

**NUMERICAL SIMULATION OF GROUNDWATER FLOW
FOR WATER RIGHTS ADMINISTRATION IN THE
CURRY AND PORTALES VALLEY UNDERGROUND
WATER BASINS, NEW MEXICO**

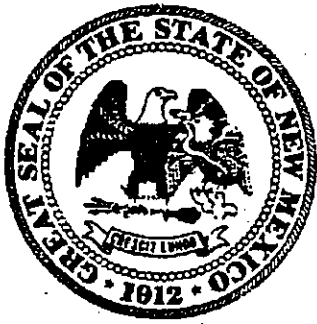
by

Ghassan R. Musharrafieh and Linda M. Logan



**New Mexico Office of the State Engineer
Technical Division Hydrology Bureau
Report 99-2**

March 1999



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ABSTRACT

The New Mexico Office of the State Engineer (OSE) declared the Portales and Curry County Underground Water Basins in 1950 and 1989, respectively. Located in eastern New Mexico, the basins are hydrologically connected. The primary aquifers in the study area are the Ogallala Formation and more recent sediments representing reworked Ogallala sediments. Water resource administrative guidelines adopted for Portales Valley expired in 1996 and guidelines have not been adopted for Curry County. At the request of the OSE's Water Rights Division, the Hydrology Bureau developed a groundwater flow model of the basins to be used for developing new administrative guidelines and for evaluating water right applications.

The two-dimensional (one layer aquifer system), finite-difference numerical model of the Southern High Plains groundwater flow system may be used to compute water level declines due to current and future groundwater diversions. Since the basins are in close hydrologic connection with adjacent lands within the state of Texas, the model extends eastward to simulate the impacts of these out-of-state wells upon the basin. This report documents model development, data sources, and model calibration. The calibrated model was used to compute the future water level declines and the remaining saturated thickness due to the continuation of existing levels of water use. Impacts of Texas withdrawals upon New Mexico were also assessed.

Model development involved estimating aquifer parameters within a plausible range so that simulated water levels match observed water levels. Aquifer parameters were estimated using a two phase trial and error procedure. In the first phase, a steady-state (pre-development) model of groundwater flow conditions was used to constrain the relationship between the model parameters; recharge, hydraulic conductivity, evapotranspiration, and natural discharge. In the second phase, or transient calibration, historical changes in water levels from 1909 to 1990 were used to evaluate the specific yield. For each phase, parameters were estimated within a plausible range of values and a simulation run was performed. Simulated water levels were compared to observed water levels to check if an acceptable match is obtained. If the difference between observed and simulated water levels is unacceptable, aquifer parameters were adjusted and another simulation was performed. The process continued until 53% of simulated steady-state water levels were within 10 feet of the observed, pre-development water levels. However, in the area of primary groundwater development in New Mexico, 71% of the simulated water levels are within 10 feet of the observed levels. Transient calibration was terminated when simulated drawdown rates matched observed drawdown rates in selected wells at the end of the simulation period.

Sensitivity analyses were performed to study the effects of changes in estimated hydraulic conductivity, recharge rate, and specific yield on simulated water levels. Aquifer parameters tested were varied while others remained unchanged from the standard value. Model response to variations in hydraulic conductivity and recharge rates was tested for pre-developed conditions. The response of the aquifer to variation in specific yield was tested for transient conditions. Changes in hydraulic conductivity, recharge rates, or specific yield increased the difference

between observed and simulated water levels.

Projected simulations to the year 2040 were performed to determine whether the current saturated thickness of the Ogallala Formation and related sediments could sustain the current production rates. The 1990 simulated saturated thickness in the basin vary from one place to another depending on the historical withdrawal rates and the pre-development saturated thickness of the Ogallala Formation. The average remaining saturated thickness in 1990 (the end of the transient simulation) was less than 100 feet in most parts of New Mexico and Texas. Historical withdrawals de-watered most areas in Portales Valley and Curry County, New Mexico, as well as parts of Bailey County in Texas, particularly west of Muleshoe. Actually, concentrated pumping in Curry and Roosevelt Counties in New Mexico as well as Bailey County in Texas will de-water large portions of the most productive areas of the basin as early as the year 2010. Parts of the basin where the remaining 1990 saturated thickness is less than 50 feet will not sustain continued pumping beyond the year 2000 or 2010. Under current withdrawal rates, current water level declines of 2.5 and 3 to 5 feet per year were simulated in Roosevelt and Curry Counties respectively. Similar rates of water level declines (3-5 feet per year) were simulated in Texas. Thus, with the current remaining saturated thickness of less than 100 feet in most areas, the Ogallala Formation will not sustain the current production rates to the year 2040. De-watered areas will increase to cover large portions of Curry and Roosevelt Counties in New Mexico and most of Bailey and Parmer Counties in Texas. However, it should be mentioned that these projections are based on the estimated withdrawal rates for current conditions. In New Mexico, these rates are approximately 79% of the permitted water rights in the basin.

The effects of Texas pumping on New Mexico water levels were examined by performing a historical simulation with no pumping in New Mexico. Simulated drawdowns indicated that Texas pumping induced 5 to 50 feet drawdown in New Mexico. The largest drawdowns are along the state line (about 50 feet) and decreased to less than 5 feet as we moved away from the state line into New Mexico. Texas pumping induced approximately 5 feet drawdown in the vicinity of Clovis and 6 to 8 miles east of Portales.

The effects of New Mexico pumping on Texas water levels were also examined by performing a historical simulation with no pumping in Texas. New Mexico pumping induced 35 to 50 feet drawdown in Texas near the New Mexico-Texas state line and decreased to 5 feet 3 to 5 miles east of the state line. Thus, the greatest effects from pumping in Texas, New Mexico, or both are observed in areas adjacent to the New Mexico-Texas state line.

A calibrated model is a valuable tool for water rights administration and an aid to planning efforts. As a tool for water rights administration, the model can provide an estimate of the amount of water remaining for appropriation and calculate the effects of proposed wells upon existing water rights. As an aid to local planning efforts, the model can be used for calculating regional effects due to current pumpage, and for evaluating different predictive scenarios due to changing future groundwater pumpage in both New Mexico and Texas.

I. INTRODUCTION

Curry County and Portales Valley in Roosevelt County are declared underground water basins located in the northern portion of the Southern High Plains of eastern New Mexico. They are unconfined in the southwestern portion of the High Plains Aquifer System (High Plains aquifer) that underlies 174,000 square miles in parts of Colorado, Kansas, Nebraska, New Mexico, Oklahoma, South Dakota, Texas, and Wyoming. In this report, reference to the Curry-Portales Basin includes Curry County, Portales Valley in Roosevelt County, and parts of Quay County in eastern New Mexico; and Parmer, Bailey, and parts of Deaf Smith Counties in western Texas. The model simulates groundwater flow in the unconfined Ogallala Formation and related sediments of Tertiary and Quaternary age which locally comprise the High Plains aquifer.

Because of the importance of the aquifer to the High Plains states, many regional as well as local studies have been implemented to better understand the geology and hydrogeology of the basin. In the late 1970's a regional multi-disciplinary study of the High Plains Ogallala Aquifer was initiated by the U.S. Department of Commerce (High Plains Associates, 1982). Locally, studies began as early as the 1930's in Portales Valley when the New Mexico Office of the State Engineer recognized the aquifer's limitation and its importance to the local economy (Theis, 1932). These studies concluded that excessive groundwater diversions will lead to aquifer mining and result in large reductions in irrigated acreage.

In the Curry-Portales Basin, irrigated agriculture uses 97% of the produced water and represents about 25% of New Mexico's irrigated lands (Wilson, 1992). Groundwater mining of the aquifer started as early as the 1920's in extensively irrigated areas and continues through the present. To protect existing water rights and to ensure orderly development of the limited resource, Portales Valley and Curry County were declared underground water basins (May 1, 1950, and on August 31, 1989, respectively) by the New Mexico Office of the State Engineer. In declared basins, new wells are subject to a regulatory, permitting process. Administrative guidelines adopted in 1956 for the Portales Valley permitted limited mining of the aquifer. Under these guidelines, pumping effects and aquifer life expectancy were computed by Theis non-equilibrium formula. Uniform values for transmissivity of 40,000 and 100,000 gpd/ft and a specific yield between 0.15 and 0.2 were used in the formula depending on the location. Guidelines have not yet been adopted for Curry County.

Anticipating the need for updating the existing administrative guidelines for Portales Valley, and developing new guidelines for Curry County, the OSE's Water Rights Division requested that the Hydrology Bureau develop a groundwater flow model for the basins. The model will be used in evaluating pending and future water rights applications. Existing groundwater flow models developed for the area are either too simple in their representation of the aquifer or too regional in scope to be used to predict local aquifer response to existing and proposed groundwater diversions. Future system response to applied stresses can best be predicted through the use of a simulation model developed specifically for detailed local conditions.

This report documents the development of a two-dimensional, finite-difference model of the High Plains aquifer in the Curry County and Portales Valley and the adjacent area in west Texas (Curry-Portales Basin). Relevant work of previous investigators on the High Plains aquifer is summarized, but the emphasis is on the data base used to calibrate the model, and the ability of the model to reproduce observed pre-development water levels and subsequent (1909-1990) changes in water table elevation.

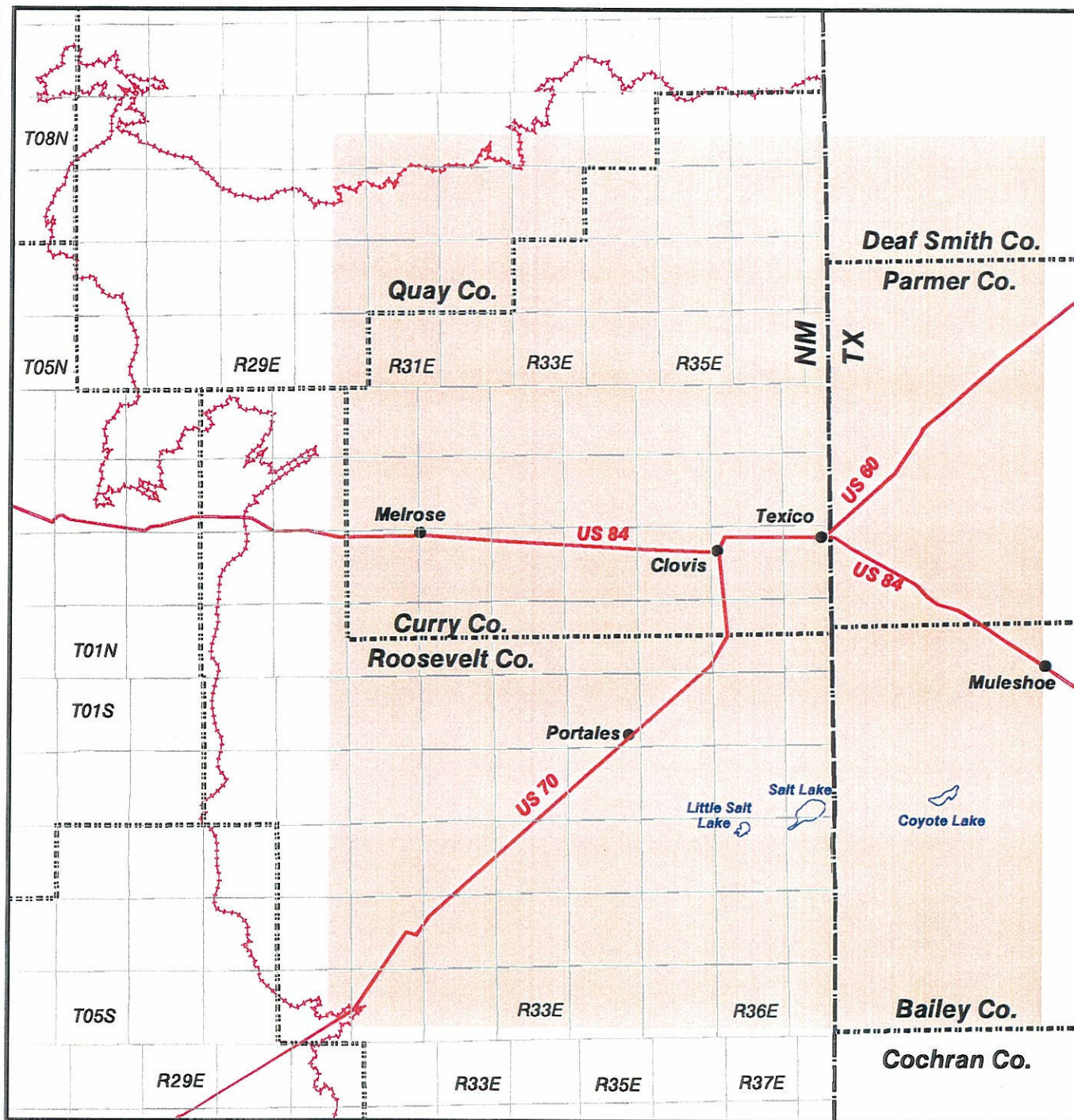
II. CURRY-PORTALES BASIN

A. Study Area Description.

The Curry-Portales Basin encompasses an area of 4059 square miles. It lies within two New Mexico declared underground water basins in the Southern High Plains of eastern New Mexico and includes portions of western Texas. From west to east, area boundaries extend from western Curry and Roosevelt Counties, New Mexico, eastward 18 miles into western Texas in parts of Bailey, Parmer, and Deaf Smith Counties. From north to south, area boundaries extend from the northern rim of the high mesa in Curry County to the northern half of Roosevelt County. In this report, references to Curry County include portions of Quay County. References to Roosevelt County, or Portales Valley refer to the northern half of Roosevelt County. Figure 1 shows the extent of the modeled groundwater basin in eastern New Mexico and western Texas.

Three regional drainage basins divide the Curry-Portales Basin, the Texas Gulf Basin, Arkansas-White-Red Basin, and a small portion of the Pecos River Basin (Wilson, 1992). This topographically high plateau or high mesa rises more than 300 feet above stream eroded valleys of the Pecos and Canadian rivers that bound this region on the north and west. Prominent escarpments of caliche that mark the mesa boundaries are referred to as "caprocks" (Fenneman, 1931). Regional slopes are gentle, about 2 to 20 feet per mile to the east-southeast with relatively flatlying grasslands covering most of the surface. Topographic relief on the mesa is low and broken only by a few wide but shallow arroyos, abandoned stream cut valleys, and depressions called playas. Groundwater levels in most areas are more than 50 feet below the land surface and do not discharge into playas except in few places where the water table is close to the land surface. Within these playas, saline lakes form wetlands for water fowl, the largest being the Salt Lake (Grulla National Wild Life Reserve) in the Portales Valley. Across the central model region lie the sand hills, a band of eolian (wind blown) dunes ranging from 2 to 10 miles wide that trend in a southeasterly direction. There are no perennial streams in the region.

In the Curry-Portales Basin, the geologic and depositional history of the Southern High Plains performed a vital role in the subsequent distribution of saturated zones within the aquifer. Triassic and Cretaceous age rocks were eroded in the early Tertiary leaving incised paleovalleys



0 2 4 6 Miles

Legend

- City
- the "Caprock"
- US Highway
- Township/Range Line
- State Line
- County Line
- Study Area

Figure 1: Study Area for Curry-Portales Basin



and bedrock highs (Nativ, 1988). After this early Tertiary erosional event, the emerging Southern Rocky Mountain uplift to the west shed sediment and runoff forming a series of Ogallala age fluvial-fans that filled the older incised paleovalleys and covered the bedrock ridges. The Triassic erosional surface resulted in radical variations in thickness of the overlying Ogallala Formation ranging from zero where the Ogallala Formation edges out against Triassic uplands to as much as several hundred feet in paleovalleys. The saturated Ogallala Formation is more than 200 feet thick in some areas of eastern Curry County and western Texas. Late Tertiary fluvial processes stopped when Ogallala streams were diverted by the subsiding Pecos and Canadian river valleys.

Overlying the Ogallala fan deposits, the Quaternary age Blackwater Draw Formation dominated the sedimentation processes. Extensive areas were covered by sheets of windblown sediments (eolian) deposited under arid to semiarid climatic conditions with concurrent development of prominent zones of caliche. Source areas for eolian sediments were from flood plains on the abandoned Ogallala braided streams and the Pecos and Canadian river flood plains. During this time, the abandoned Ogallala stream valley (Portales Valley) was filled to about 100 feet with deposits of reworked Tertiary Ogallala and eolian sediment. Eolian processes deposited loess (fine-grained silty deposits) in the northern areas and southeasterly trending sand dune belts accumulated over Tertiary and Triassic sediments. Wind erosion formed several hundred depressions or playas in the Blackwater Draw Formation throughout the Curry County region. The reworked Ogallala of the Portales Valley form a locally important hydrostratigraphic unit of the High Plains aquifer while the sand dunes and playas may be an important component of local recharge. Detailed descriptions of the sedimentation processes, sediments, and depositional environment are given by Seni (1980), Hawley, (1984), Reeves, (1984), Gustavson and Finely, (1985), and Gustavson and Winkler, (1988).

These geologic units are of varying hydrologic importance and comprise the High Plains aquifer. The areal extent of these units vary between erosional remnants of low permeability Triassic and Cretaceous strata that are exposed in a few areas but are typically buried, to extensive sheets of Tertiary age Ogallala sediments that blanket most of the area, and channels of Quaternary reworked Ogallala that fill Ogallala age valleys (Gutentag et. al, 1984). Principle hydrostratigraphic units are the Ogallala Formation in Curry County, and the Quaternary valley fill deposits in Portales Valley.

Groundwater is the only reliable source of fresh water in the area. Surface water sources (in playas and arroyos) are intermittent and not dependable. There are no perennial streams and of the many playas, only a few form small playa lakes to provide wetlands for water fowl. Playa lakes tend to be intermittent and even the largest, Salt Lake (Grulla National Wild Life Reserve) may revert to a mud flat during some hot dry summers. Two major surface drainages dissect the northern area, Frio and Running Water Draw, that trend southeasterly, are intermittent and carry runoff from thunderstorms.

Most groundwater production in the area is from the mineralogically similar Tertiary Ogallala Formation and Quaternary reworked Ogallala of the Portales Valley. Through the

sorting process, the reworked Ogallala tends to be generally more permeable than the undisturbed Ogallala. Within the Ogallala Formation, saturated thicknesses are extremely variable, discontinuous, and commonly thin on the western and northern edge of the mesa. Near Clovis and Portales, aquifer saturated thickness is sufficient to support irrigated agriculture. Low permeability Triassic age rocks are hydraulically connected to the more permeable Ogallala Formation, but do not constitute a reliable source of water. A few discontinuous fluvial Triassic sands within the finer grained main body may provide small quantities of water of lesser quality than the Ogallala.

Groundwater from the High Plains aquifer is essential to the Curry-Portales farming community and primary population centers, Clovis and Portales. About 75% of the population of Curry County live in Clovis and 64% of the population of Roosevelt County live in Portales. Of the produced groundwater in the region, 97% is used for irrigation and the other 3% is consumed by manufacturing, industry, public supply, domestic and stock uses (Wilson, 1992). Texas, High Plains agriculture is very important and depends on groundwater from the common aquifer.

As a farming community, climatic conditions in the Curry-Portales Basin establish the amount of groundwater consumed by irrigation. Climatic conditions in the study area are characterized as semi-arid with low average annual precipitation and high average annual evaporation rates. Mean annual precipitation ranges from 13 to 17 inches per year and occurs during the time of the highest seasonal evaporation (evaporation from a free water surface) rates of 39 to 52 inches per year. Summers are usually low in humidity with clear, warm to hot, sunny days and cool nights. Winter temperatures are moderate with some severe cold spells. The average snowfall is light and the snows usually melt within a few days. Light to moderate winds prevail most of the year except for January to May when strong winds are common (Finker, 1987).

B. Groundwater Development.

Portales Valley was the first area to develop groundwater for irrigation on New Mexico's Southern High Plains. In the early 1900's farmers irrigated small tracts with water pumped from windmills and a few gasoline powered pumps. Large scale irrigation began in 1910 with the construction of an electric power plant. After this initial growth, numerous farms failed due to the lack of farming experience. Irrigated lands declined in the next few years to the point that the number of remaining farms were not significant enough to be included in the 1919 census. In the late 1920's and early 1930's large scale irrigation resumed in the Portales Valley (Theis, 1932) and continued to expand in the Muleshoe District of Texas on the eastern boundary of study area (White et. al., 1946). It was not until the early 1950's that large scale irrigated agriculture began to develop throughout the study area and the growth of irrigated farms moved westward into the study area. Drought conditions of the 1950's accelerated groundwater development throughout the Southern High Plains adding to the demands of increasing irrigated lands. Within the study area, the Texas agricultural industry was as much as 60% less than New Mexico Curry-Portales

area until the 1950's when this trend reversed. Texas now has about 30% more irrigated lands than New Mexico.

Accelerated groundwater development has lead to mining of the aquifer in Texas and New Mexico. Water levels have declined more than 150 feet in Texas (Texas Department of Water Resources, 1992) and 125 feet in some areas of New Mexico (Chudnoff and Logan, 1994) where extensive withdrawals are required for agricultural and public supplies.

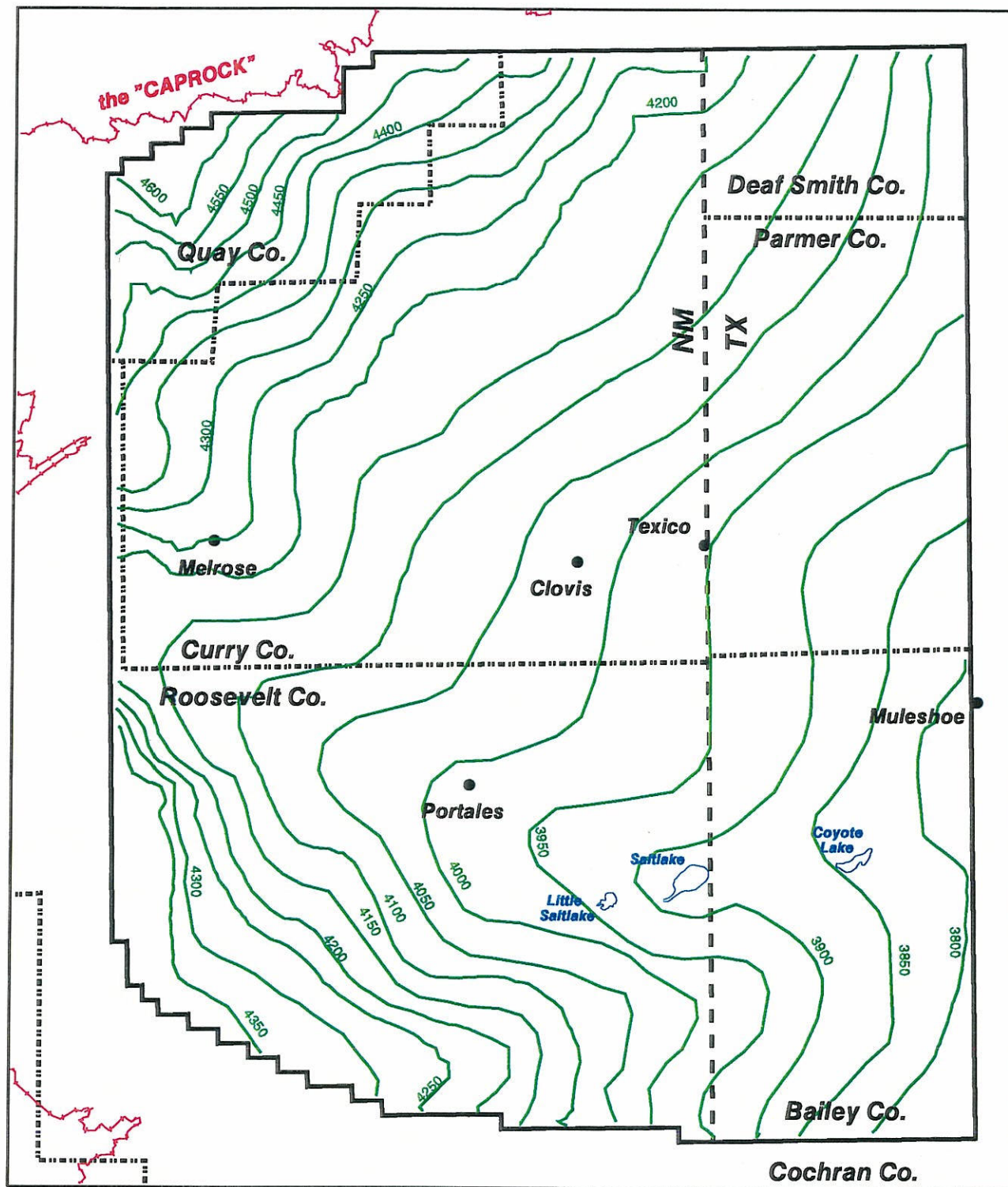
C. Hydrological Setting and Aquifer Characteristics.

1. Water Table.

The Curry-Portales Basin aquifer is unconfined and groundwater flow is in a southeasterly direction, subparallel to regional topographic and depositional gradients. Pre-development groundwater flow gradients vary from one place to another and average 10 to 12 feet per mile. Figure 2 shows the continuous nature of the pre-development water table (Akin and Jones 1979) that appears to be influenced primarily by the higher permeability of the Portales Valley alluvium. Figure 3 shows land surface elevations of the modeled area.

Pre-development saturated thickness of the Ogallala Formation is shown in Figure 4 (Akin and Jones, 1979). This figure illustrates the great variability in the thickness of the saturated material through out the region and buried paleovalleys and ridges in the area. In Curry and Roosevelt Counties, the pre-development saturated thickness varies from less than 50 feet in the west to approximately 200 feet near the New Mexico-Texas state line. Pre-development saturated thickness increases to the east. In some parts of Bailey and Parmer Counties in Texas the pre-development saturated thickness exceeds 200 feet.

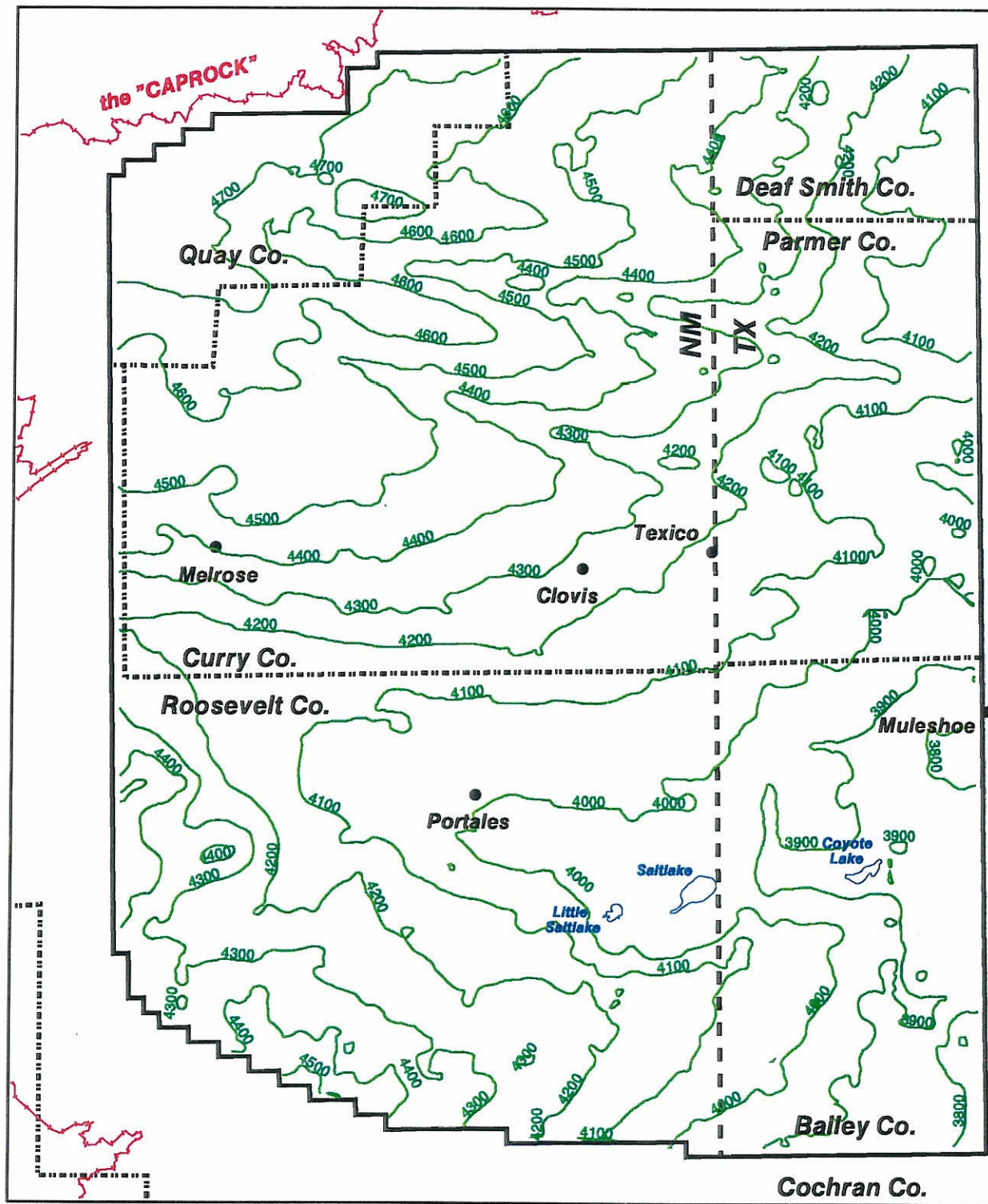
Fifty years of groundwater development has created numerous cones-of-depression that reversed groundwater flow gradients around extensively irrigated areas. Figure 5 illustrates the changes in water-table elevation through an intensely irrigated area south of the Curry-Roosevelt County line from pre-development time to 1992 (Chudnoff and Logan, 1995). This cross section shows where groundwater depletions have increased water table depths and altered the regional flow gradient. Because of continued water level declines and the aquifer's importance to the agricultural based economy in the Curry-Portales area, water levels are monitored by the USGS and OSE on a regular schedule. Additional historical and recent water-table information may be found in summaries by Chudnoff and Logan (1995), Nativ and Smith (1987), Gutentag and Weeks (1980), Akin and Jones (1979), and Cronin (1969).



Legend

- Model Boundary
- State Line
- County Line
- Water-Table Elevation

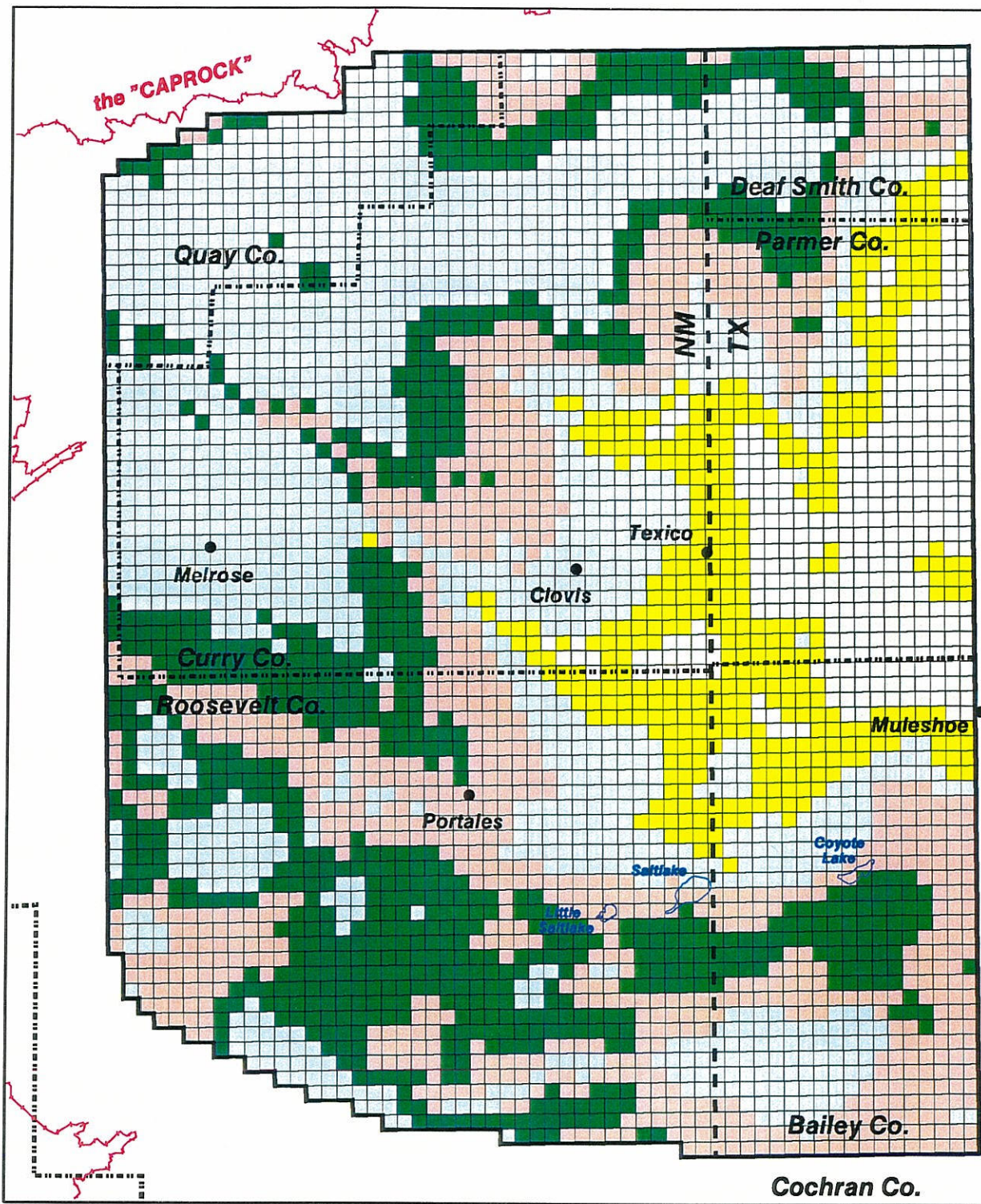
**Figure 2: Observed
Pre-development
Water Table Elevation (ft)**



Legend

- Model Boundary
- State Line
- County Line
- Land Surface Elevation

**Figure 3: Land Surface
Elevation (ft)**



Legend



- Model Boundary
- State Line
- County Line

Figure 4: Pre-Development Saturated Thickness of the Curry-Portales Basin (ft)

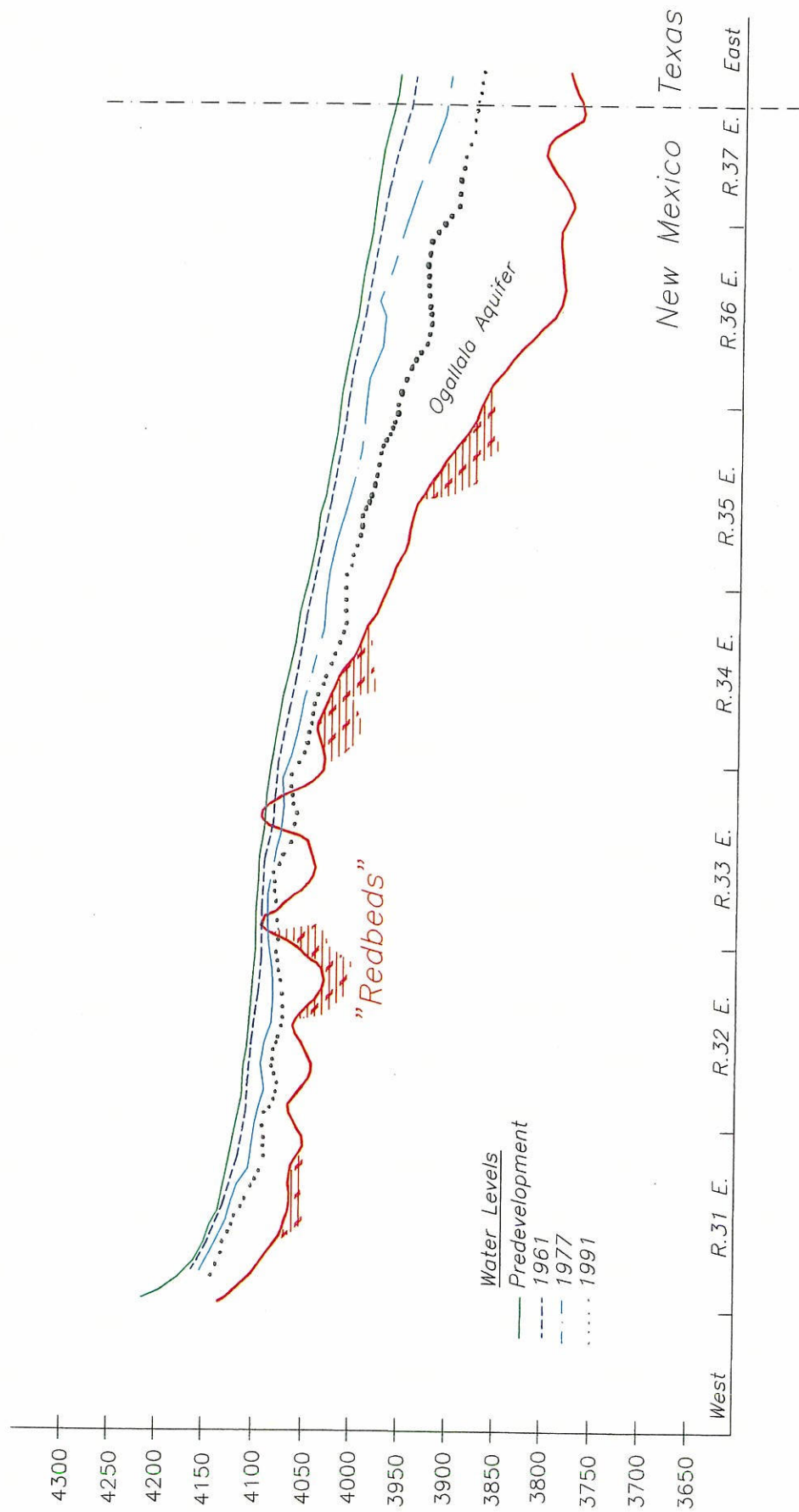


Figure 5. West to East Water Table Profiles from Pre-Development to 1991 Through Township 1 North, Range 30 to 37 East (Chudnoff and Logan, 1995)

2. Hydraulic Conductivity.

Two fundamental characteristics of an aquifer system are its hydraulic conductivity, a measure of the aquifer's ability to transmit water, and the thickness of the water bearing material. The product of these two is aquifer transmissivity, a parameter that is defined as the rate at which water is transmitted through a unit width of aquifer material under a unit hydraulic gradient. Because the aquifer in the Curry-Portales Basin is unconfined and the saturated thickness varies depending on pumping locations and rates, aquifer hydraulic conductivity and not aquifer transmissivity will be discussed in the following paragraphs.

Several investigators have tried to estimate hydraulic conductivity values for the Ogallala Formation in Texas and New Mexico using different methods. Theis (1934), using multiple-observation well tests, obtained conductivity values between 150 and 195 feet per day in the Portales Valley. Galloway and Wright (1968) estimated regional conductivity values between 50 and 214 feet per day for the Portales Valley. They based their work on the geology of the area and previous work by Theis (1934) and Mantei and others (1966). Howard (1954), based on specific capacity tests, estimated conductivity values between 8 and 92 feet per day east of Clovis. Regional hydraulic conductivity values derived from an analog model that included the Portales and Clovis areas average 112 feet per day (Mantei and others 1966).

Estimated hydraulic conductivity for the Ogallala Formation in Lea County, New Mexico, showed lower values than those for Curry and Roosevelt Counties, although these aquifers are part of the High Plains aquifer. Theis (1939) reported conductivity values for the Ogallala Formation between 41 and 48 feet per day from work done by Nye (1932) near Hobbs, Lea County. Theis (1939) performed laboratory analysis on samples collected in and near Lea County and obtained a conductivity range between 2 and 17 feet per day. Havens (1966) obtained an average conductivity value of 43 feet per day in Lea County.

Based on drillers logs and test cores, Knowles and others (1984) estimated (and mapped) aquifer hydraulic conductivity between 3 and 262 feet per day for the Ogallala Formation in Texas. Myers (1969) obtained a range between 10 and 230 feet per day (average 56 ft/day) from aquifer tests from 19 wells in 6 counties in Texas. Near Amarillo, Texas, aquifer tests yielded conductivity values between 21 and 25 feet per day (Moulder and Frazor 1957). Near Plainview, Texas, Cronin and Wells (1960) obtained similar conductivity values (13 to 23 feet per day) from long term aquifer tests. In general aquifer tests in Texas resulted in a wide range of conductivity values (3 to 262 feet per day). Table 1 summarizes hydraulic conductivity estimates reported in the literature. The table is organized based on the tests nearest the area of interest.

Triassic red-beds play an important role in the study area, yet their hydrologic influence is poorly understood. Triassic red-beds underlie the area and form buried valleys and ridges within the High Plains aquifer, and are the target for the maximum depth of wells in the area. Studies by Nativ and Smith (1987) indicated that there is no upward flow from the basement Triassic to the Ogallala. However, horizontal flow through Triassic highs was mapped by Akin and Jones (1979)

	Hydraulic Conductivity Range (ft/d)	Average Value (ft/d)	Method	Tests	Data Source
Portales Valley, New Mexico	150-195		Aquifer Tests	2 Mult- Observ.	Theis, 1934
Portales Valley, New Mexico	53 - 214		Averaged Others Work		Galloway and Wright, 1968
Curry County, New Mexico	8 - 92	52	Specific Capacity	14	Howard, 1954
Lea County, New Mexico	41- 48		Aquifer Tests		Theis, 1939
Lea County, New Mexico	2 - 17		Laboratory		Theis, 1939
Lea County, New Mexico	43		Estimated		Havens, 1966
Amarillo, Texas	21 - 25		Aquifer Tests		Moulder and Frazor, 1957
Plainview, Texas	13 - 23		Aquifer Tests		Cronin and Wells, 1960
Lovington, New Mexico	13		Laboratory Analysis		Havens, 1966
Southern High Plains, Texas	10 - 230	56	Aquifer Tests	19	Myers, 1969
Southern High Plains, Texas	3 - 262	30	Drillers Logs, Cores	41 Test Holes	Knowles and others, 1984
Estimated Hydraulic Conductivity from Analog and Digital Model Calibration					
Portales Valley, New Mexico		112	Analog Model		Mantei and others, 1966
Lea County, New Mexico	10 - 175	40	Digital Model		McAda, 1984
Lea County, New Mexico	10-120		Digital Model		Musharrafiieh and Chudnoff, 1999
Southern High Plains, New Mexico and Texas	10 - 150		Digital Model		Luckey and others, 1986

Table 1. Tertiary Ogallala Hydraulic Conductivity Values from Various Sources.

and is apparently continuous. Triassic valleys and highs locally influence the saturated thickness of the High Plains aquifer. On the western boundary of the basin, saturated Triassic provides inflow to the hydraulically connected Ogallala Formation. Triassic sands within the siltstone dominant formation are erratic but are a source of water locally. Wells are generally low yielding and the water quality is poor containing three to ten times more total dissolved solids than the water in the Ogallala Formation (USGS analyses in Galloway and Wright, 1957).

There are no reported aquifer tests conducted within the Triassic in the study area. Only a few tests were conducted in Texas, near the northern rim of the Southern High Plains, where some commercial irrigation is possible. These tests were used to estimate the plausible range of hydraulic conductivity values in the Triassic red-beds of New Mexico (Table 2). Hydraulic conductivity values for the Cretaceous Formations are also shown in Table 2. Cretaceous rocks in the study area play a minor role and their presence do not significantly influence hydraulic conductivities estimates in the model.

Study Area	Hydraulic Conductivity Range (ft/d)	Formation Age	Method	Tests	Data Source
Midland, Texas	15-69	Trinity Group, Cretaceous	Aquifer Tests	8 Tests	Myers, 1969
Deaf Smith and Swisher Co., Texas	4-38	Dockum Group Triassic	Aquifer Tests	8 Tests	Dutton and Simpkins, 1986

Table 2. Cretaceous and Triassic Hydraulic Conductivity Values.

3. Specific Yield.

The specific yield (Sy) of an unconfined aquifer is the volume of water that can be drained by gravity from the saturated sediments per unit drop in the water table per unit area. Specific yield may be determined in-situ by aquifer tests, in the laboratory from cores, or calculated by dividing the volume of water pumped by the volume of sediments de-watered.

No aquifer tests to determine aquifer specific yield have been reported for the Curry-Portales Basin. Alexander and others (1943) computed a specific yield values of 0.14 and 0.15 near Plainview and Hereford, Texas, by dividing the net amount of water pumped from 1938 to 1943 by the volume of de-watered sediments. Specific yields of 0.09 to 0.16 and 0.11 to 0.14 were obtained from aquifer tests conducted near Amarillo, Texas (Moulder and Frazor, 1957), and Plainview, Texas (Cronin and Wells, 1960) respectively. In Lea County, Havens (1966) used

the same methodology used by Alexander and others (1943) and obtained a specific yield value of 0.24 from data collected from 1930 to 1955. McAda (1984) in the Lea County Model calibration, used a specific yield range between 0.12 and 0.25 for the Ogallala Formation based on these and other studies. Table 3 summarizes specific yield estimates for the Ogallala by various methods. There are no known specific yield estimates reported for the Triassic.

4. Groundwater Recharge and Discharge.

Precipitation is the primary source of recharge to the aquifer. Infiltration of areal precipitation is volumetrically the most important source of groundwater recharge for the High Plains aquifer with some contribution from runoff into playas, and arroyos (Nativ and Riggio, 1990; Dvoracek and Black, 1973). The precipitation fraction that actually infiltrates and recharges the aquifer is strongly controlled by the semi-arid climatic conditions in the study area that are characterized as low average annual precipitation and high average annual evaporation rates. Consequently, most precipitation evaporates within a few hours of falling or may be held near the soil surface to replenish the soil moisture.

Recharge contributions from playas is not yet understood. Playas are common throughout the Southern High Plains and capture as much as eighty to ninety percent of rainfall. Early investigators believed that most of this water evaporates and very little will infiltrate (Dvoracek and Black, 1973). Later studies indicated that infiltration through playas is important and higher recharge rates are observed near playas. Subsequent studies in the 1990's have shown that playas have different recharge profiles depending on their sedimentologic characteristics and moisture content at the time of a runoff event. In general, playas are focused recharge points where infiltration rates are initially very high and decrease exponentially with time. Mullican et. al., (1994) modeled several recharge scenarios and found that focused (playa) recharge gave similar results to areal recharge. Nativ and Riggio (1990) found that areal recharge estimates varied by two orders of magnitude due to different interpretations of the importance of playa lakes contributions to aquifer recharge. More information on aquifer recharge could be found in Dvoracek and Black, 1973; Keir and others, 1984; Stone, 1984; Wood and Osterkamp, 1984; Stone and McGurk, 1985; Scanlon et. al., 1994; Zartman et. al., 1994.

Recharge may occur from precipitation runoff through ephemeral stream channels and arroyos. Caliche layers are common in arroyos in this area and were first thought to prevent infiltration (Cronin, 1964). Later investigators showed that recharge is possible through fractures and root tubes in the caliche layer (Allison and others, 1985; Stone and McGurk, 1985).

A fraction of diverted groundwater may return to the aquifer as irrigation return flow depending on the depth to the water table and the loading rate. In a chloride mass balance test, Stone (1984, 1990) determined that 10.32 inches/year of irrigation return flow reaches the water table. This amount is 344 (Stone 1984, 1990) times more than natural recharge through precipitation although it is not an external recharge source. Knowles and others (1984) found

Study Area	Specific Yield Range	Average	Method	Number of Tests	Data Source
Portales Valley New Mexico	0.20		Reported by Mantei		Galloway and Wright, 1968
Lea County, New Mexico	0.28		Grain Size Tests		Theis, 1934
Lea County, New Mexico		0.24	Calculated Q/de-watered		Havens, 1966
Lovington, New Mexico	0.38		Laboratory Test	1, Repacked Sample	Havens, 1966
Southern High Plains, Texas	0.15 - 0.20 Undisturbed	0.29 Compacted	Laboratory Test	8	Barnes and others, 1949
Southern High Plains, Texas	0.072 - 0.195			19	Myers, 1969
Southern High Plains, Texas	.07 - .19	.16	Grain Size, Cores	41 Test Holes	Knowles and others, 1984
Plainview, Hereford, Texas	0.14 - 0.15		Calculated Q/de-watered		Alexander and others, 1943
Amarillo, Texas	.09 - 0.16		Aquifer Tests		Moulder and Frazor, 1957
Plainview, Texas	0.11 - 0.14		Aquifer Tests		Cronin and Wells, 1960
Specific Yield Estimates from Analog and Digital Model Calibrations					
Portales Valley, New Mexico	0.20		Analog Model		Mantei and others, 1966
Lea County, New Mexico	0.12-0.25		Digital Model		McAda, 1988
Lea County, New Mexico	0.12 to 0.24		Digital Model		Musharrafieh and Chudnoff, 1999
Southern High Plains, Texas	0.15		Digital Model		Luckey and others, 1986

Table 3. Specific Yield Values from Various Sources for the Ogallala Formation.

that irrigation return flow moved at a rate of less than 1 foot per year and only in areas with a shallow water table would irrigation return flow be a possible source of aquifer recharge.

Table 4 summarizes groundwater recharge estimates by various investigators. Recharge estimates ranged from 0.01 to 1.71 inches per year depending on the method used. Details of recharge investigations using these methods are described by Nativ and Riggio (1990); Stone and McGurk (1985); Stone (1984); Klemm (1981); Barnes and others (1949); Theis (1937). In spite of all the recharge studies, total recharge in the area is commonly unknown, difficult to estimate, and frequently determined through model calibration efforts.

Natural groundwater discharge in the study area occurs only through evapotranspiration when the water table is near the surface. We are not aware of any evapotranspiration estimates reported for the area which introduces uncertainty in the parameter. Increasing groundwater development has increased the depth to the water table and reduced evapotranspiration from the aquifer. The only three playas that apparently intercepted the water table before aquifer development began are Salt Lake, Little Salt Lake, and Coyote Lake. These lakes cover less than two square miles in the southern part of the study area and the volume of water discharged through these lakes is very small. Artificial groundwater discharge (extraction) constitute a major source of discharge in the basin and will be discussed in detail in the modeling section.

III. MODEL DESCRIPTION AND DEVELOPMENT

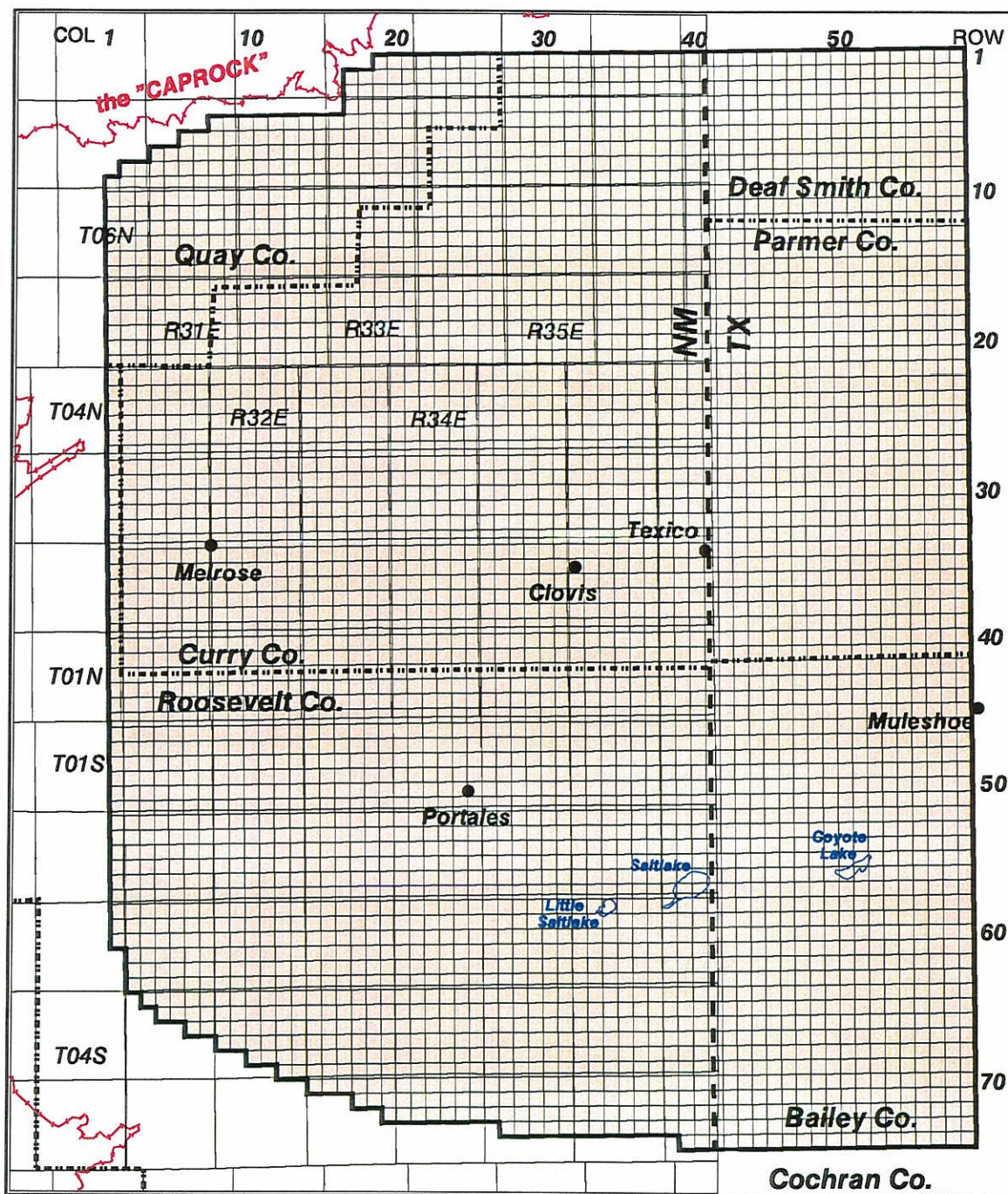
A. Model Discretization.

The modeled area covers 4059 square miles, of which 2727 square miles are in eastern New Mexico and 1332 square miles are in western Texas. In New Mexico, the modeled area covers the two underground water basins declared by the Office of the State Engineer, Curry and Portales Valley Underground Water Basins. The 1332 square miles into Texas (18 miles east) are introduced to minimize boundary effects on the area of interest and to study the effect of large groundwater diversions in western Texas on water levels in New Mexico. As mentioned earlier, the two declared basins in New Mexico and the extension into Texas will be referred to as the Curry-Portales Basin.

The three-dimensional finite difference model code developed by McDonald and Harbaugh (1988) was used to simulate two-dimensional flow in a single layer aquifer in the Curry-Portales Basin. Model development began by discretizing the study area into a one square mile finite difference grid (cell) for numerical solution. For this problem, the area was divided into 74 rows and 58 columns with the New Mexico-Texas state line in column 40 of the model grid as shown in Figure 6. The number and the size of cells in the grid is usually determined by the purpose of the model. A finer mesh grid requires more input data and longer processing time and is recommended for models designed to compute drawdowns in the vicinity of a well or to simulate

Location	Type	Recharge (in/yr)	Method	Source
Portales Valley, New Mexico	Areal	< 0.5	Darcy's Law	Theis, 1937
Curry Co., New Mexico	Areal, Arroyo, Sand Hills	.01 - .48	Chloride mass-balance	Stone and McGurk, 1985
Curry Co., New Mexico Blackwater Draw Formation	Areal	0.03	Chloride mass-balance	Stone, 1990
Curry Co., New Mexico	Zonal, Sand Hills	0.17	Chloride mass-balance	Stone, 1990
Curry Co., New Mexico	Playas	0.48	Chloride mass-balance	Stone, 1990
Curry Co., New Mexico	Irrigation Return	10.32	Chloride mass-balance	Stone, 1990
Lea County, New Mexico		0.8		Havens, 1966
Southern High Plains, Texas	Playa	3.24	Isotopes	Nativ and Riggio, 1990
Southern High Plains, Texas,	Areal	0.05	Isotopes	Nativ and Riggio, 1990
Southern High Plains, Texas	Sand Hills	0.833	Neutron logs	Knowles and others, 1984
Southern High Plains, Texas	Areal	0.058 - 0.571	Neutron logs 22 sites	Knowles and others, 1984
Recharge Estimates from Digital and Analog Model Calibration				
Portales Valley, New Mexico	Areal	1.71	Analog	Mantei and others, 1966
Ogallala Formation	Areal	0.354	Digital	Mullican and others, 1994
Blackwater Draw Formation	Areal	0.236	Digital	Mullican and others, 1994
Playas	Playas	8.62	Digital	Mullican and others, 1994
Lea County, New Mexico	Aerial	0.1 - 0.5	Digital	Musharrafieh and Chudnoff, 1999
Lea County, New Mexico	Aerial	0.35-0.45	Digital	McAda, 1988
Southern High Plains, New Mexico	Aerial	0.086	Digital	Luckey and others, 1986
Southern High Plains, New Mexico	Arroyo	1.03	Digital	Luckey and others, 1986

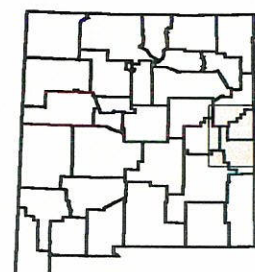
Table 4. Groundwater Recharge Estimates by Various Investigators.



Legend

- City
- the "Caprock"
- Township/Range Line
- Model Boundary
- State Line
- County Line
- Model Grid

Figure 6: Model Grid



stream-aquifer inter flow along specific stream reaches. For this regional model, a one mile square grid is considered adequate to calculate the average aquifer response to an applied stress to aide water rights administration.

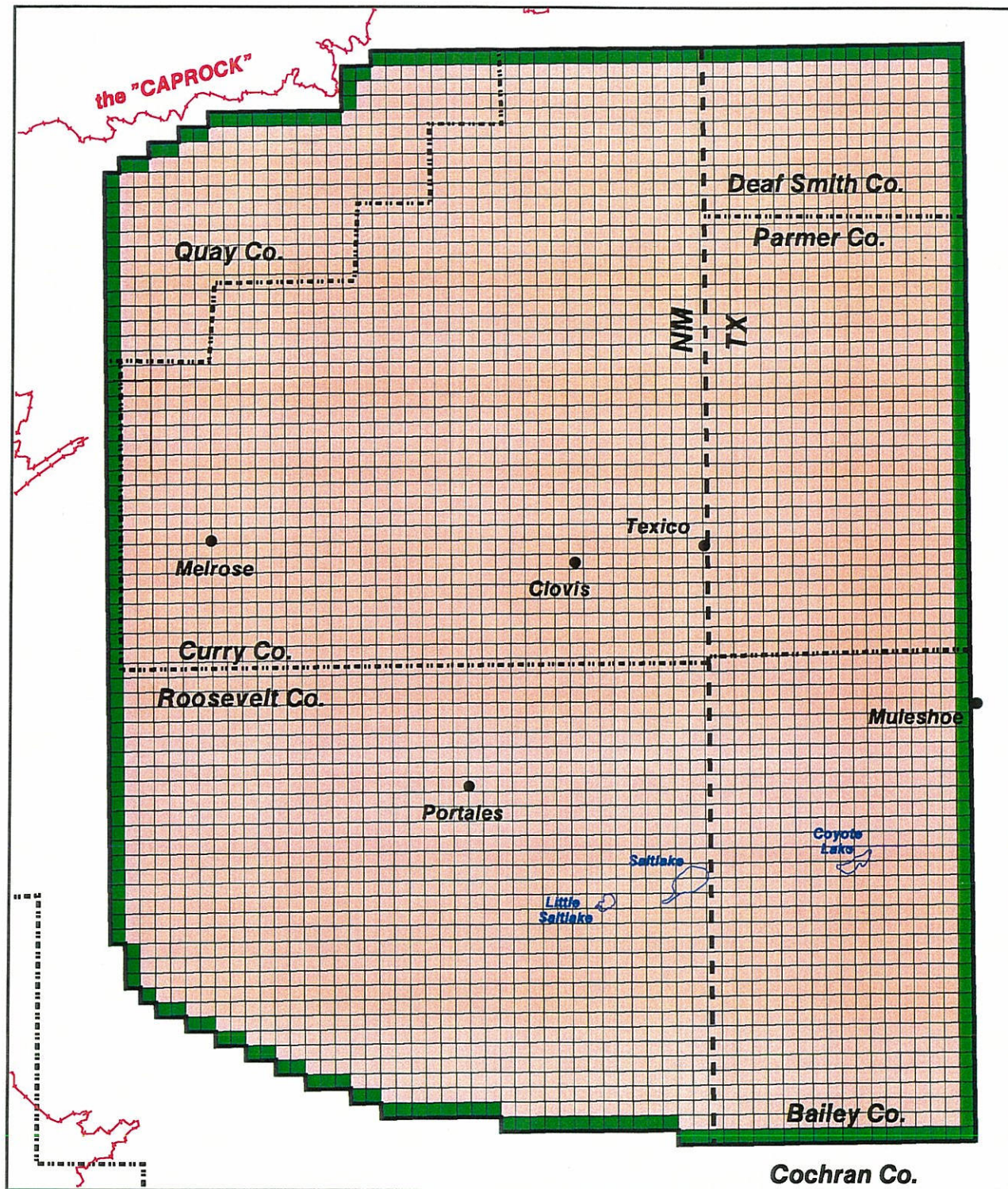
B. Steady-State Boundary Conditions.

Following model discretization, boundary conditions were defined. The types of boundaries that could be simulated depended on the proximity of the physical system to hydrologic boundaries. Hydrologic boundaries could be “no flow”, “specified or constant head”, and “specified flux”. A “no flow” boundary is specified if the flow domain is adjacent to an impermeable geological formation that prevents water from entering or leaving the system. A “constant head” boundary is specified if the basin is hydraulically connected to an adjacent water source. This type of boundary allows an unlimited amount of water to enter or leave the system to maintain a constant hydraulic head at the boundary. A “specified flux” boundary allows a specified amount of water to enter or leave the flow domain independent of the hydraulic head at the boundary. A “no flow” boundary is a special case of a specified flow boundary where the amount of water entering or leaving the flow domain is zero.

In model development for steady-state simulations, boundaries to the north, south, east, and west sides of the basin are modeled as “constant head”. The reason for simulating constant heads at the northern, southern, and western boundaries is to compute subsurface inflow to the aquifer originating as areal recharge from outside the modeled area. The eastern boundary in Texas is simulated as a “constant head” to compute subsurface flow discharging into Texas. Figure 7 shows steady-state boundary conditions simulated by the model.

The bottom impermeable layer is assumed to be the bottom elevation of the Ogallala Formation, or the top elevation of the Triassic red-beds. This is modeled as a “no flow” boundary. As discussed earlier, Triassic red-beds range in depth from few feet in some areas to several hundred feet in others. They are hydraulically connected to the Ogallala Formation but have a much lower hydraulic conductivity. In very few areas (mainly in the northwest of the basin) where the Triassic red-beds outcrop the onlapping Ogallala Formation or the saturated thickness of the Ogallala Formation is very small, the continuity of subsurface flow is maintained through the system by assuming an equivalent Ogallala saturated thickness equal to the transmissivity of the Triassic red-beds divided by the hydraulic conductivity of the Ogallala Formation. Areal recharge through the upper boundary, or land surface, is simulated as “specified flux”. Evapotranspiration (ET) from the aquifer through the upper boundary, is modeled as “head dependent flux”. Actual ET from the aquifer varies depending on aquifer material, extinction depth (depth from the ground surface to the water table at which ET ceases), and the maximum ET rate. An extinction depth of 25 feet was selected for the model. Recharge and ET through plays were not treated differently in the model.

After boundary conditions are defined, observed steady-state water levels, bottom



Legend

- Constant Head Cell
- Variable Head Cell
- Model Boundary
- State Line
- County Line

**Figure 7: Pre-development
Boundary Condition
Simulated in the Model**

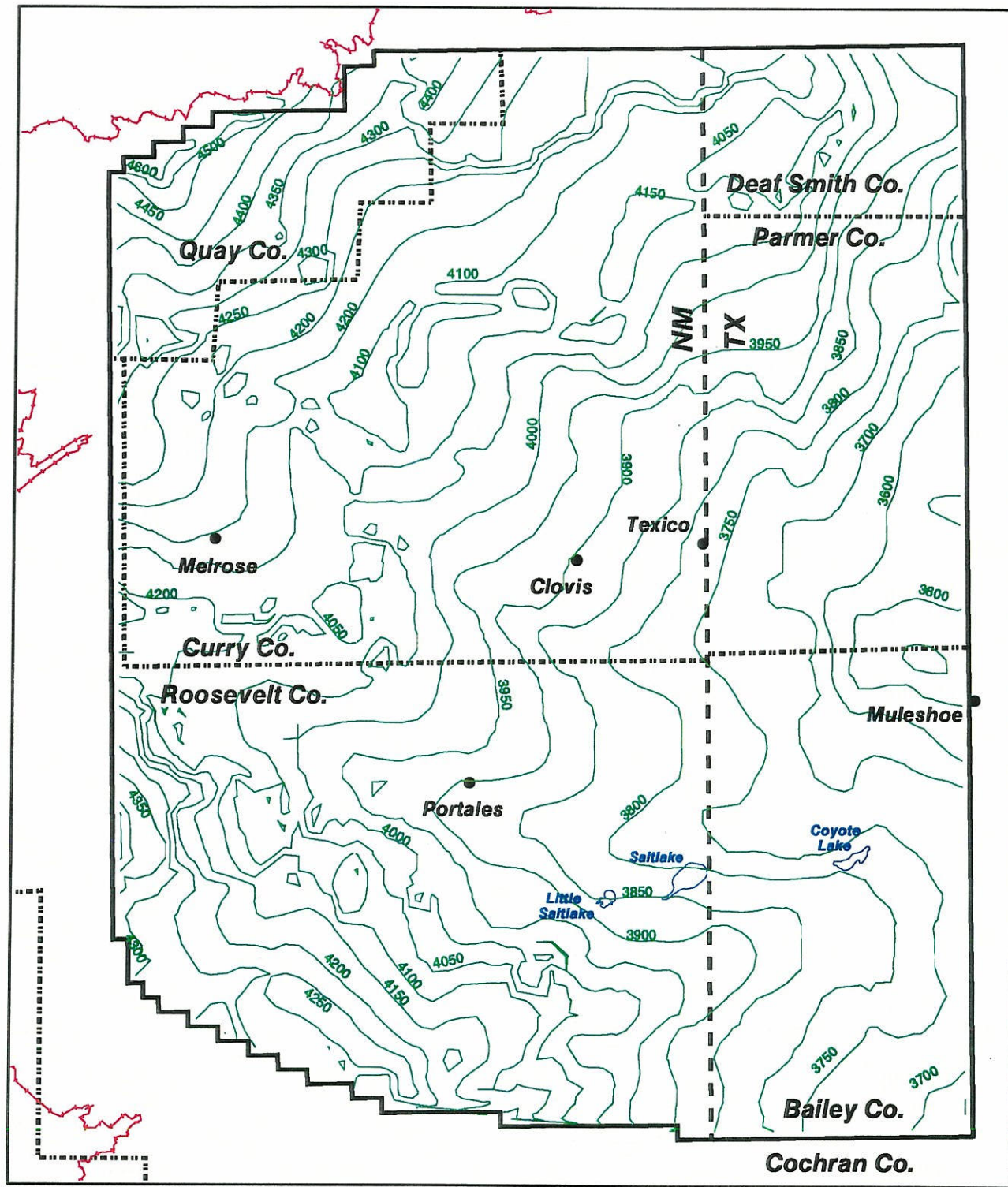
elevations (Figure 8), and land surface elevations (Figure 3) are compiled from Akin and Jones (1979) maps and input into MODFLOW files. Aquifer hydraulic conductivity, recharge rate, and evapotranspiration rate were estimated from the range of plausible values and also compiled. Estimates of specific yield are not necessary for steady-state simulations.

C: Transient Simulations Boundary Conditions.

Transient simulations were performed to estimate aquifer specific yield. Simulated steady-state water levels were used as starting heads and simulated subsurface flows (Appendix A) through constant head cells were used as specified fluxes at the northern, southern, and western boundaries in subsequent transient simulations. As mentioned earlier, a "specified flux" boundary allows a constant amount of water to enter and leave the basin independent of the hydraulic head at that boundary. Similar to the steady-state model, recharge through the top boundary is simulated as "specified flux", evapotranspiration is simulated as "head dependent flux", and the bottom boundary is simulated as "no flow".

Constant head boundary cells on the eastern model boundary in Texas presented a problem as to how they should be represented in the transient model. It is not possible to represent the boundary as a "constant head" because water levels are changing due to pumping along this boundary in Texas. It is also not possible to represent it as a "no flow" boundary because it is clear that extensive pumping from inside and outside of the modeled area will induce flow across the boundary. A possible representation of this boundary is a "specified flux" boundary. Due to lack of sufficient field data in Texas, the amount of specified flow across the eastern boundary was assumed. Simulated steady-state flows across the eastern boundary could not be used as specified fluxes in the transient model because withdrawals along this boundary reduce subsurface discharge through this boundary. However, two scenarios were simulated to determine whether the assumed discharge through the eastern boundary have any effect on simulated water levels in the area of interest, New Mexico. Initially, in the transient model (standard model), the assumption was made that the amount of water discharging through this boundary equals the simulated pumping rates along this boundary. In other words we did not attempt to discharge more water through this boundary than the simulated historical pumpage. However, to test if the discharge rate has any effect on simulated water levels in New Mexico, we performed two additional simulations with different discharge rates. These simulations are discussed in the following section.

In the first simulation, no pumping was simulated in column 58 (no flow is allowed to discharge through the eastern boundary). Simulated water levels in New Mexico were compared with simulated water levels from the standard model which allowed discharge eastward out of the model boundary. The difference in water levels from the two simulations was less than 0.3 feet along the New Mexico-Texas state line (column 40 of the model grid) and decreased as we moved away from the state line into New Mexico. In the second simulation, no pumping was allowed in columns 57 and 58. This means that the amount of water discharged from the eastern



Legend

- Model Boundary
- State Line
- County Line
- Bottom Elevation

**Figure 8: Aquifer
Bottom Elevation (ft)**

boundary is further decreased. Again water levels from this and the standard simulation did not differ by more than 0.3 feet along the New Mexico-Texas state line and decreased as we moved away from the state line into New Mexico. It was concluded that the discharge rate through the eastern boundary does not have a significant effect on simulated transient water levels in New Mexico.

D: Transient Simulations Well Diversions.

For the transient simulations, estimated historic pumping data for the Curry-Portales Basin from 1909 to 1990 were compiled into 8 stress periods in a well file. More than 97% of the water used was for irrigation and the remaining 3% was for public water supply, industrial, and domestic and stock use (domestic and stock usage are less than 0.1% of the total diversion). Luckey and others (1986) estimated that pumpage for irrigation totaled more than 97% of all groundwater diversions on the Southern High Plains and chose to neglect all other uses in their model. For transient simulations all uses, including domestic and stock wells were summed for each model cell.

The vast majority of wells are not metered in the modeled area requiring that water use be estimated for most model cells. For the first 4 stress periods, an initial attempt was made to estimate historical pumpage for irrigation by using indirect methods which relate pumping plant energy costs to well discharge rates. This method proved to be unsatisfactory because assumptions needed to evaluate and distribute this scant data added a great deal of uncertainty. We finally decided to estimate agricultural use from irrigated acreage, crop irrigation requirement (CIR), and on-farm delivery efficiency. Data for other groundwater uses came from reports and maps that described water use in the basin.

For the New Mexico area of the model, and early Texas data, estimates for irrigation diversions were made for each stress period by multiplying the irrigated acreage for each model cell by the average crop irrigation requirement (CIR) and on-farm delivery efficiency. Estimates of CIR varied from 0.95 to 2.1 acre-feet per acre as cropping patterns changed from the 1950's to the 1990's (Henderson and Sorensen, 1968; Wilson 1992). The amount of diverted water equals the CIR divided by on-farm efficiency (82%, Wilson 1992). Studies have shown that not all irrigation diversions are depleted from the aquifer and that net aquifer depletions are less due to irrigation return flow. The amount of return flow is unknown and highly variable according to Stone (1984) and Klemt (1981) who emphasized the high variability in irrigation return flow amounts and the uncertainty associated with return flow estimates. To avoid this additional uncertainty, we assumed for the water use estimates that the average aquifer depletion equals the crop irrigation requirement of 2.00 acre-feet per acre for all stress periods.

Historical irrigated acreages used to estimate water use for the 1909's to 1960 (stress periods 1-5) for New Mexico and Texas were derived from early reports and distributed into model cells using maps by Theis (1932), White and others (1946), Mabe (1951), Cooper (1960),

OSE Curry Map (1961), Dinwiddie (1963), and Mantel and others (1966). Irrigated acreage distribution and estimated water use for the 1960's to 1980's (stress periods 6 and 7) were based on OSE inventories and SCS maps (USDA-SCS, 1978, 1979; OSE unpublished data, 1978). Estimates for the 1980's and 1990 (stress period 8) were based on OSE inventories, maps and areal photographs that provided accurate mapping and crop-type analysis (USDA-SCS, 1978, 1979; OSE unpublished data, 1979, Koogler and Pooler, 1992). Figure 9 and Table 5 show the total amount of groundwater diverted from the modeled area during each stress period from New Mexico and Texas.

Early Texas irrigated acreages before 1959, were distributed using maps from several reports. In general these irrigated areas were not near the state line and had no effect on New Mexico at that time (White, et al, 1946; Mantel, et al, 1966; and Texas Department of Water Resources, 1981). Later withdrawal estimates for the modeled areas in Texas from 1959 to 1990 were provided by the Texas Water Development Board from their Texas Southern High Plains model GWSIM-III (Knowles, 1981, and Peckham, personal communication, 1995). The Texas model discharge input files included irrigation, municipal and industrial, and domestic and stock wells. This data was grouped into 10 year time steps from 1960 to 1990 with discharge estimates based on county reports and distributed by percentages into the GWSIM-III model cell of 5364 acres. For use in the Curry-Portales Basin model, the two model grids were superimposed and the Texas data was distributed into the 1332 mile size model cells.

Well locations (or pumping centers) in New Mexico used in model development came from several sources, primarily the USGS-GWSI (1992) data base which provided extensive coverage of wells throughout the High Plains of eastern New Mexico. Earlier data not included in the USGS-GWSI data base were augmented from various reports by Cooper (1960), Howard (1954), and Theis (1932). Well locations in Texas were provided by the Texas Department of Water Resources for the years 1936 to 1990. Figure 10 shows pumping center locations used in the model.

IV. MODEL CALIBRATION AND SIMULATIONS

Steady-state and transient simulations were performed to estimate aquifer parameters so that simulated water levels match observed water levels. A trial and error cycling procedure was used to estimate aquifer hydraulic conductivity, evapotranspiration rate, and recharge rate. A cycle begins by assuming a hydraulic conductivity distribution, evapotranspiration rate, and recharge rate within their plausible ranges and a steady-state simulation was performed. Simulated steady-state water levels were then compared with observed pre-development water levels to determine if an acceptable match was obtained. If the match was un-acceptable, spatially variable hydraulic conductivity, recharge rate, and evapotranspiration rate were varied within their plausible ranges and another simulation was performed. The process continued until the 53% of the simulated water levels were within 10 feet of the observed.

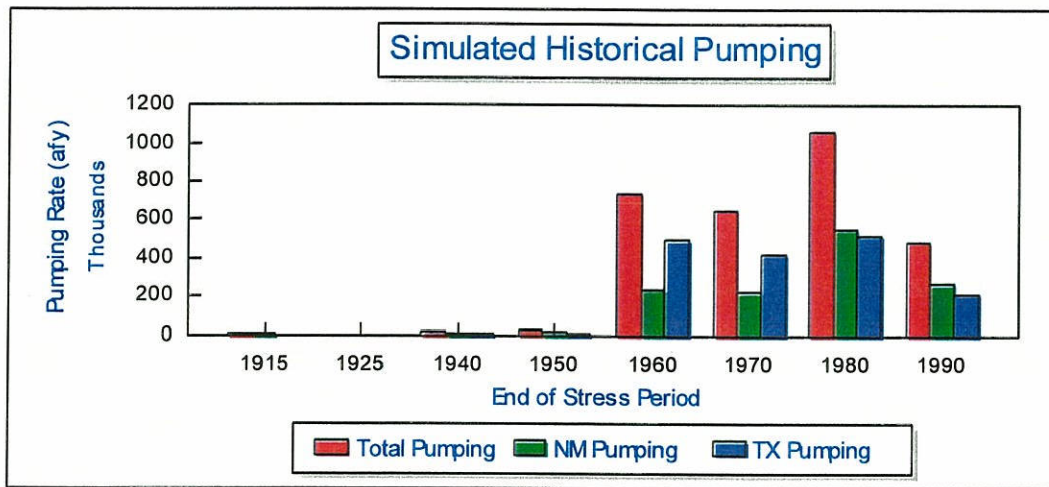
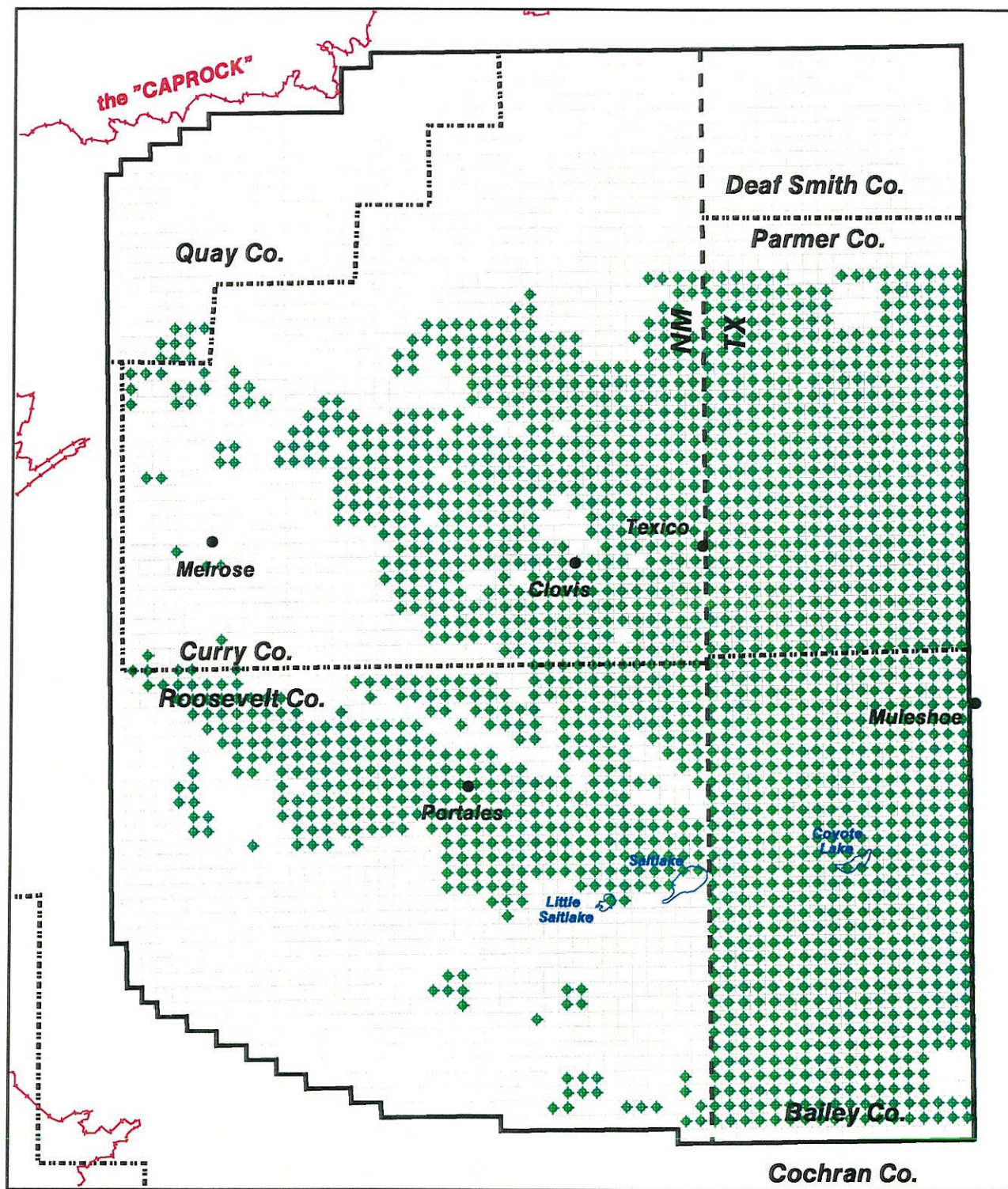


Figure 9. Water Use Simulated in the Model.

Stress Period	Total Pumping (afy)	New Mexico Pumping (afy)	Texas Pumping (afy)
1909-1915	4,219	3,985	234
1916-1925	335	101	234
1926-1940	14,170	11,504	2,666
1941-1950	24,610	15,923	8,687
1951-1960	736,286	237,619	498,667
1961-1970	646,454	229,667	416,786
1971-1980	1,061,853	545,794	516,059
1981-1990	486,810	269,151	217,659

Table 5. Water Use Simulated in the Model.



- Legend**
- Model Boundary
 - State Line
 - - - - County Line
 - ◆ Pumping Center

Figure 10: Location of Pumping Centers Simulated in the Model

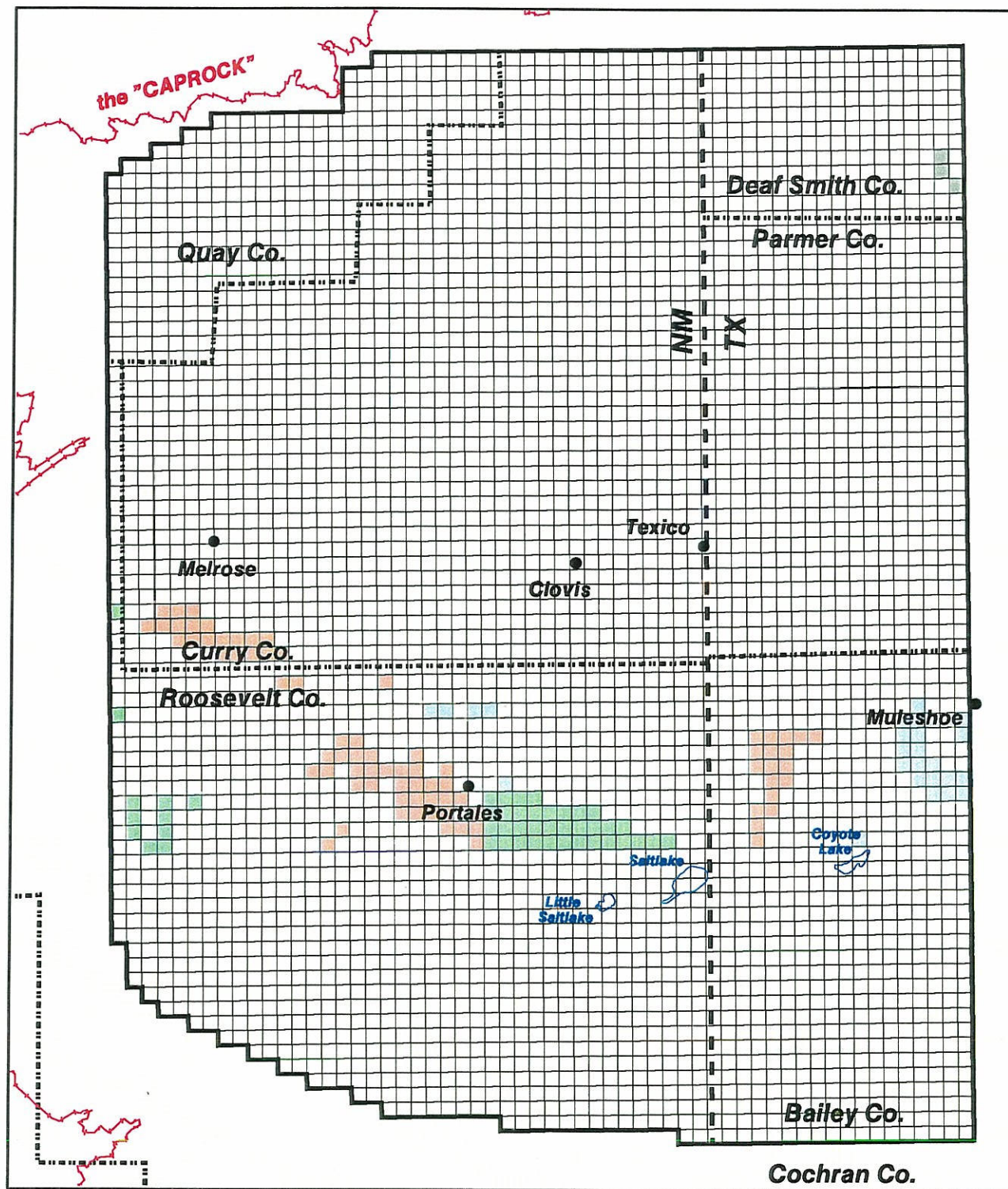
Following steady-state simulations, transient simulation were begun to estimate aquifer specific yield. During transient simulations only specific yield was varied within a plausible range to obtain an acceptable match between the rate of observed and simulated drawdown. Estimated hydraulic conductivity, recharge rate, and evapotranspiration rate were not varied in transient simulations. However, if varying specific yield within its plausible range did not yield an acceptable match, then hydraulic conductivity, recharge rate, and evapotranspiration rate were modified and a steady-state simulation is performed. The process continued until an acceptable match between simulated and observed drawdown rates were obtained.

Calibrated model parameters are shown in Figures 11 through 14. Figure 11 shows the evapotranspiration (ET) rate used in the model. Evapotranspiration occurred in areas where the water table is within 25 feet of the land surface. The maximum evapotranspiration rate varies from one place to the other and ranged from 0.1 east of Portales to 2 inches per year in other areas. The maximum simulated evapotranspiration is 2.05 inches per year. This resulted in the best match between simulated and observed steady-state water levels. A higher maximum ET rate seemed to give a better match between simulated and observed steady-state water levels in Lea County (approximately 3.5 inches/year). As mentioned earlier, actual evapotranspiration from the aquifer also varies depending on aquifer material, extinction depth (depth from the ground surface to the water table at which ET ceases), and the maximum ET rate. An extinction depth of 25 feet was selected for the model.

Figure 12 shows the distribution of areal recharge in the model. Recharge rates vary from one place to another depending on the infiltration rate of the unsaturated material at the land surface and the annual rainfall. Recharge distribution in the model ranged from 0.04 to 0.66 inches per year. Higher recharge rates are simulated along the sand hills in New Mexico and Texas and lower rates are simulated in areas to the northwest and southwest where less infiltration takes place. However, because recharge rates are poorly known, initial estimates were then modified within their plausible range to obtain a better fit between observed and simulated water levels through the model calibration process. Final recharge estimates are reasonably close to our initial expectations.

Figure 13 shows the hydraulic conductivity distribution simulated in the model. Hydraulic conductivity values ranged from 5 to 150 feet per day. Initial hydraulic conductivity distribution was specified based on subsurface geology, saturated thickness, and water table gradients. Lower conductivity values were assigned to areas having small saturated thickness and steep flow gradients. Higher conductivity values were assigned to areas having greater saturated thickness and gradual flow gradients (Portales Valley). However, initial estimates were slightly modified within their plausible range to obtain a better fit between observed and simulated water levels through the model calibration process. Final hydraulic conductivity estimates are reasonably close to our initial expectations.

Figure 14 shows the specific yield distribution simulated in the model. Specific yield values ranged from 18% to 25%. Specific yield values between 0.23 and 0.25 are simulated for a

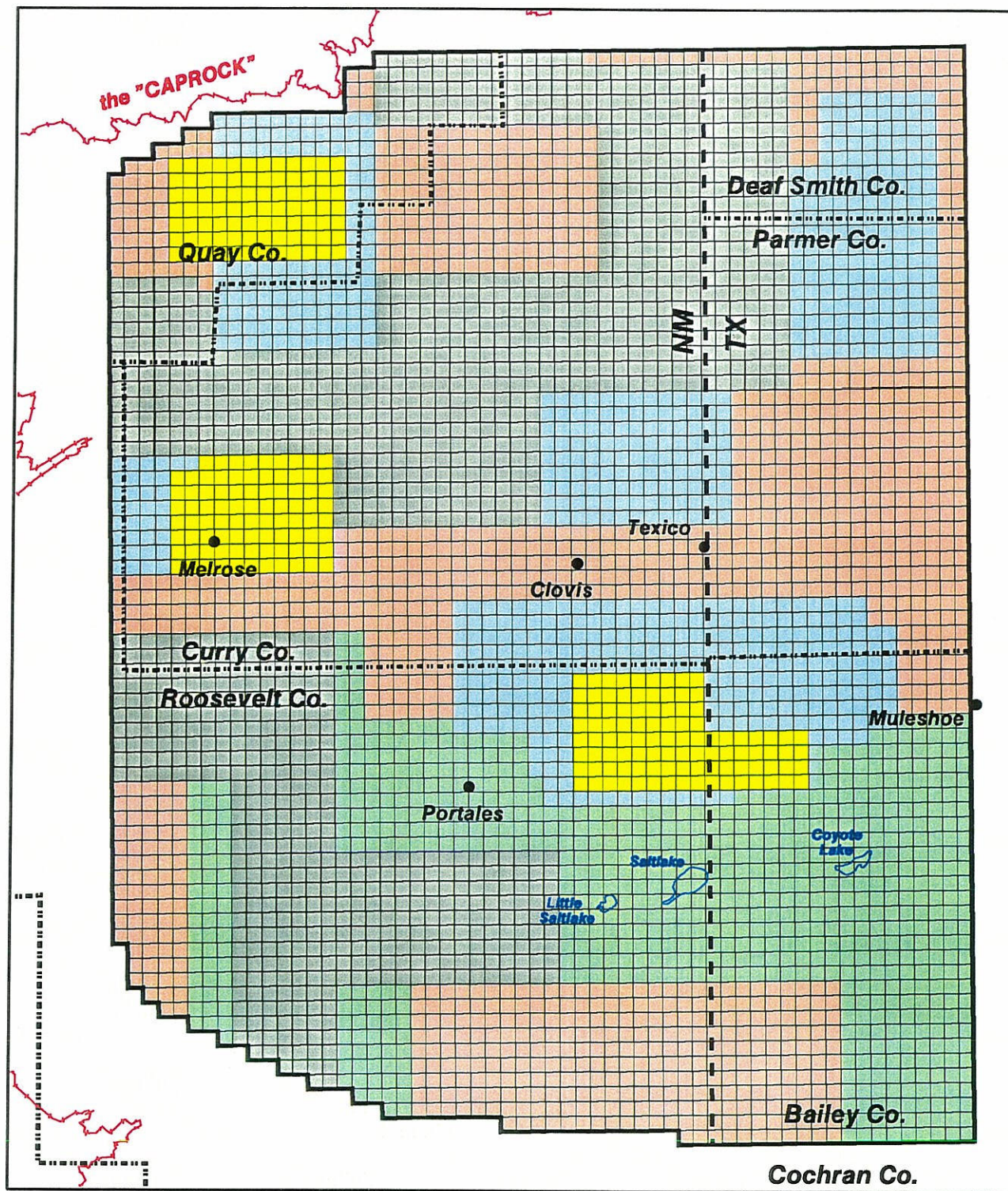


Legend



- Model Boundary
- State Line
- County Line

Figure 11: Initial Evapotranspiration Rate Simulated in the Model (in/yr)

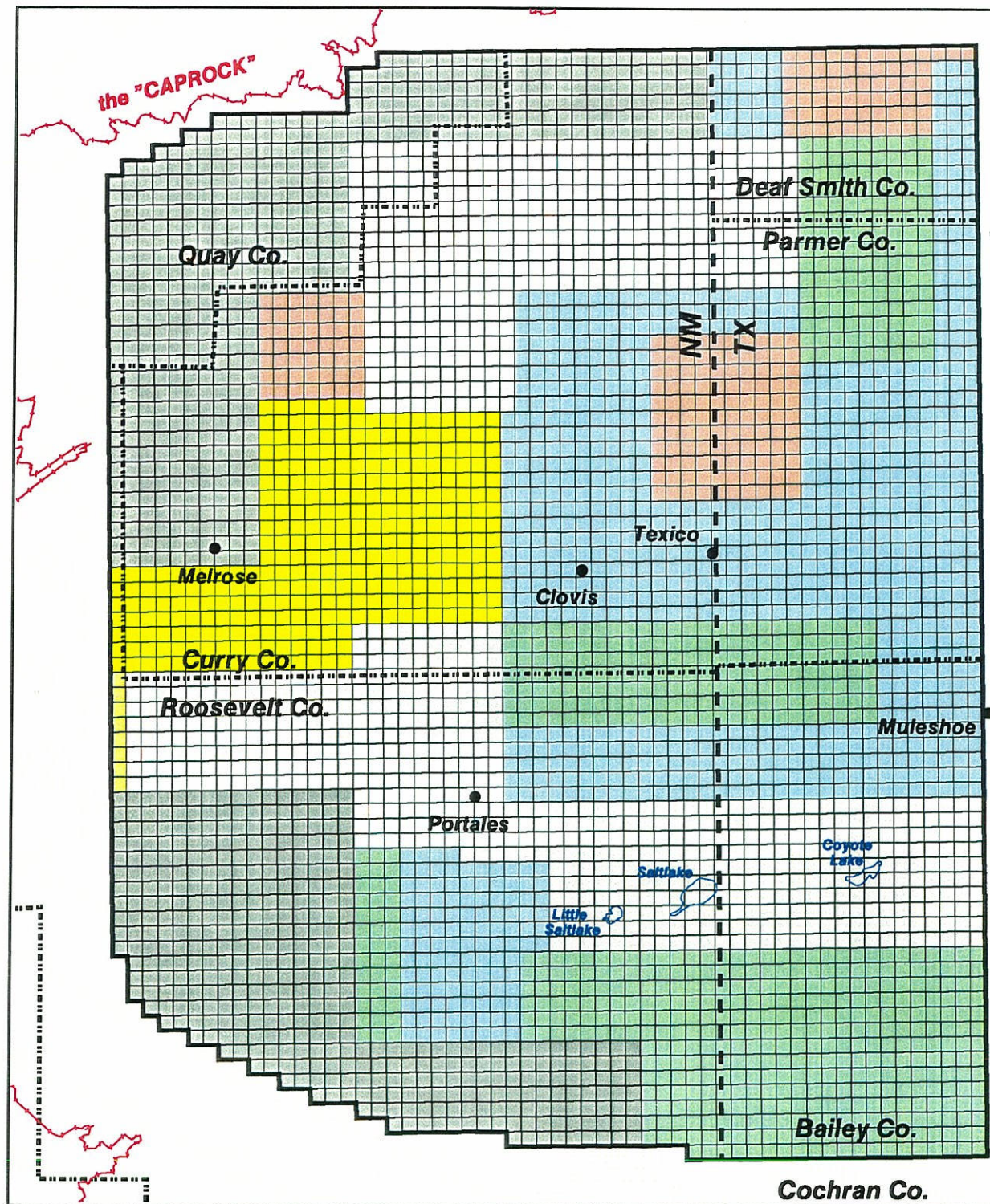


Legend



- Model Boundary
- State Line
- County Line

Figure 12: Areal Recharge Rate Simulated in the Model (in/yr)

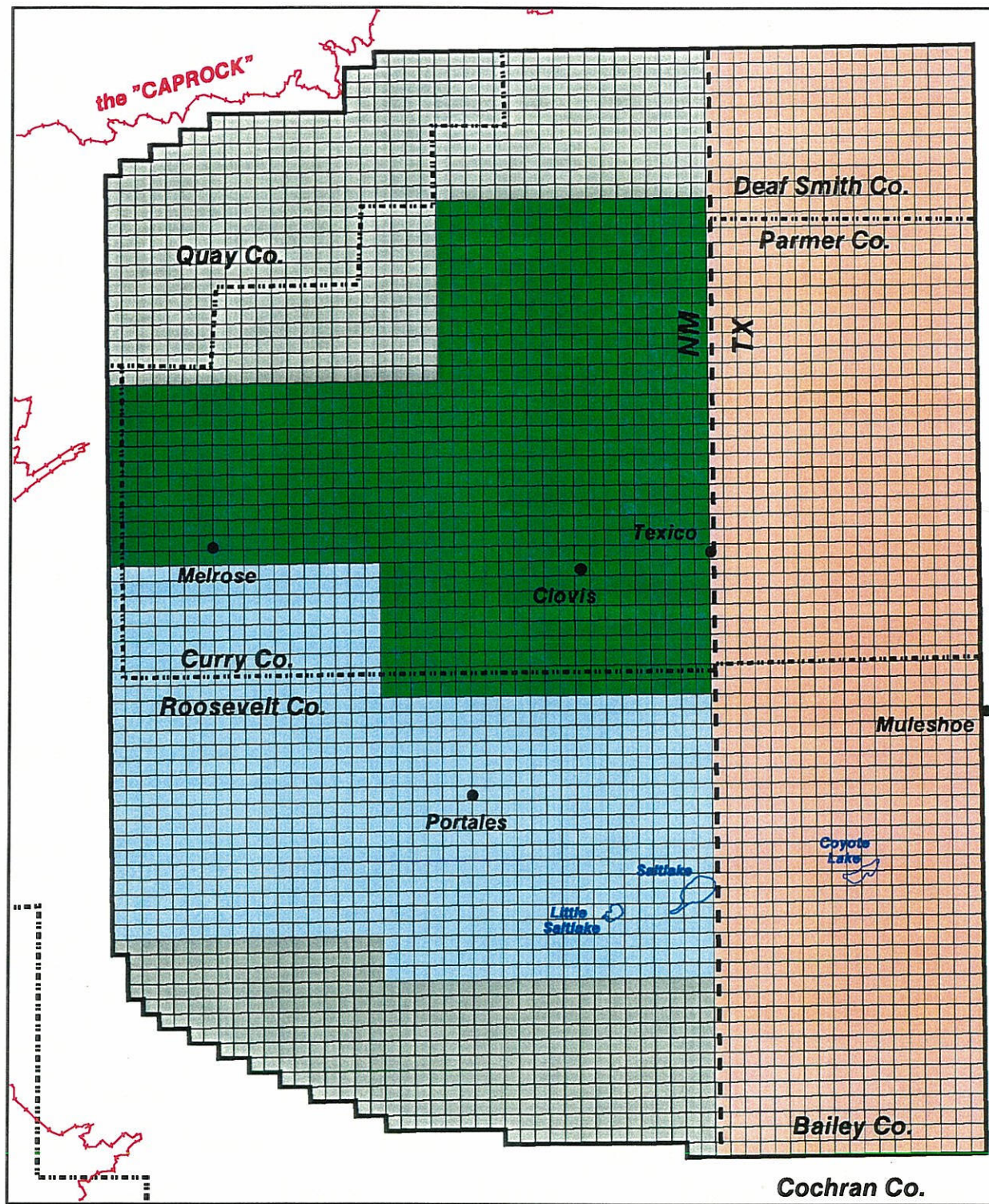


Legend

1-20	61-80
21-40	81-100
41-60	>100

- Model Boundary
- State Line
- County Line

Figure 13: Hydraulic Conductivity Distribution Simulated in the Model (ft/d)



Legend

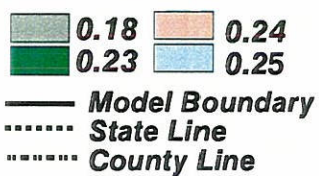


Figure 14: Specific Yield Distribution Simulated in the Model

large portion of the basin except for few areas to the southwest and northwest. Again, similar to the other parameters, specific yield values were modified within their plausible range to obtain a better fit between observed and simulated water levels.

A. Simulated Steady-State Water Levels.

Simulated steady-state and observed pre-development water levels (prior to 1909) are shown in Figure 15. Pre-development water level data from individual wells are not available. Observed pre-development water levels were obtained from Akin and Jones (1979) maps based on earlier work by the OSE and United States Geological Survey (USGS) on the Southern High Plains Aquifers. Although it is not known what year these water levels represent it is assumed that they represent pre-development or pre 1909. Theis (1932) reported that groundwater withdrawals in Portales Valley started as early as the 1900's before well records were maintained and there is a possibility that Akin and Jones pre-development water table maps did not account for drawdowns caused by this early pumping. Consequently, an error could be associated with the observed pre-development water levels as reported by Akin and Jones (1979) and used in this model calibration.

The difference between observed and simulated steady-state water levels are shown in Table 6 and Figures 16 and 17. Table 6 shows the Mean Error, Mean Square Error, and the Root Mean Square Error (RMSE) for the differences between simulated and observed water levels. These errors called the "calibration error" express the average difference between observed and simulated water levels. Table 6 shows that 53% and 77% of the difference between observed and simulated water levels are within 10 and 20 feet respectively when the whole model grid is considered. However, when Curry County and Portales Valley (the area of primary groundwater development, between rows 20 and 55, and columns 20 and 40) are considered, 71% and 92% of the differences between simulated and observed water levels are within 10 feet and 20 feet, respectively. The difference between observed and simulated water levels increased near model boundaries.

The simulated water budget for the steady-state model is shown in Table 7. Areal recharge is the most important source of water inflow to the aquifer. Subsurface recharge through constant head cell (subsurface recharge originates as areal recharge outside the model boundaries) constitutes approximately 21% of the total recharge while areal recharge within the study area represent 79%. Subsurface discharge through constant head cells and evapotranspiration constitute approximately 86% and 14% of the total discharge from the aquifer, respectively.

B. Simulated Transient Water Levels.

Figure 18 shows measured and simulated historical drawdowns for each stress period in

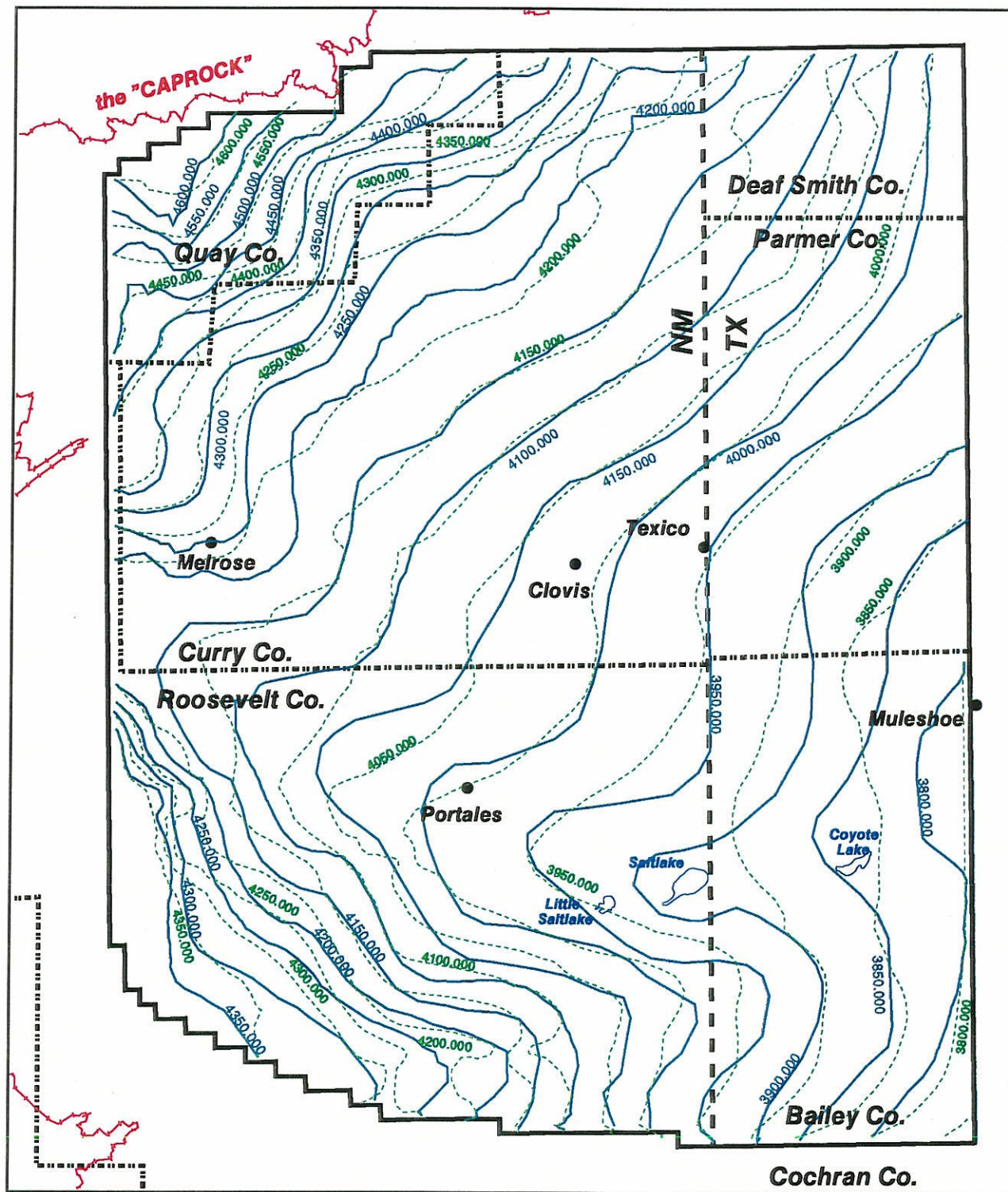


Figure 15: Simulated and Observed Steady-State Water Levels

Range of Difference Between Computed and Measured Water Levels (feet)	ALL MODEL GRID (number of cells)			AREA OF INTEREST Rows 20-55 & Columns 20-40		
	Under Pred.	Over Pred.	% of Total	Under Pred.	Over Pred.	% of Total
0-10	1255	861	53.0	333	206	71.4
10-20	435	510	24.0	87	68	20.5
20-30	227	325	14.0	40	19	7.8
30-40	91	141	6.0	0	2	0.3
40-50	44	77	3.0	0	0	
50-60	18	23	1.0	0	0	
60-70	4	2	0.1	0	0	
70-80	1	0	0.1	0	0	
80-90	0	0	0	0	0	
Mean Error (ME)		-1.5			1.7	
Mean Square Error (MSE)		12.9			7.7	
Root Mean Square Error (RMSE)		17.6			10.6	

Table 6. Frequency Distribution of the Difference Between Steady-State Simulated and Observed Water Levels.

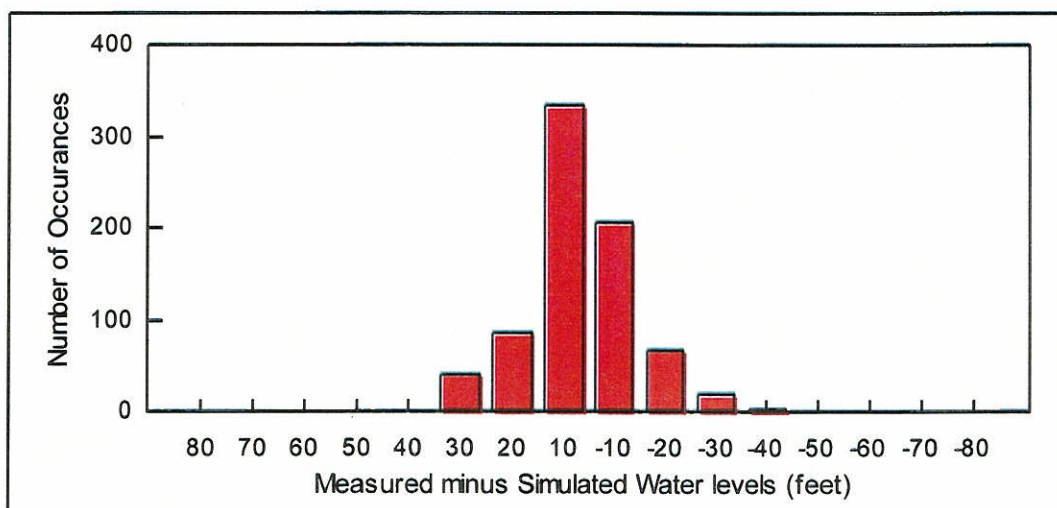


Figure 16. Simulated Steady-State Water Levels minus Observed Water Levels for the Area of Interest, New Mexico (Rows 20 to 55 and Columns 20 to 40).

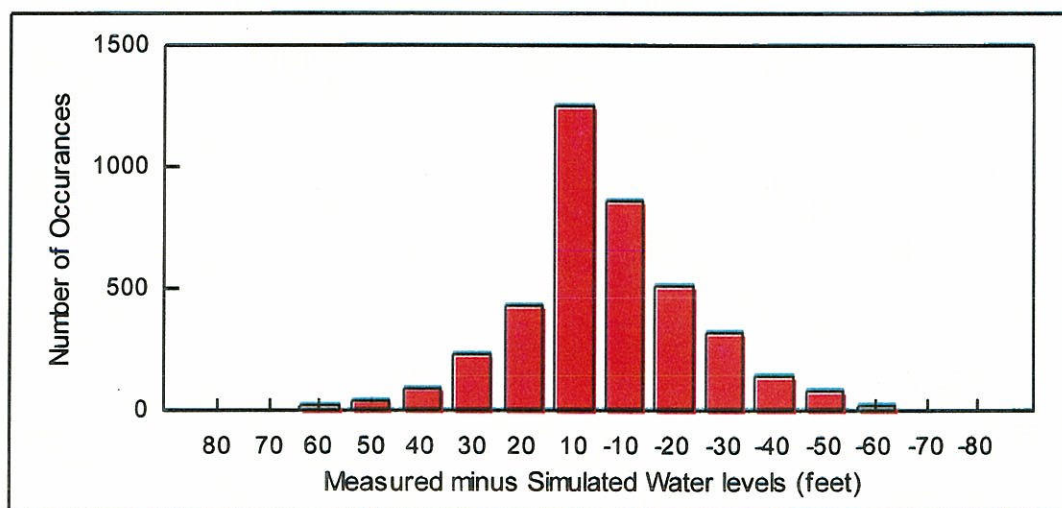


Figure 17. Simulated Steady-State Water Levels minus Observed Water Levels for the Model Area.

Source	Recharge	Discharge
Change in Storage	0	0
Constant Head	11,807	48,731
Areal Recharge	45,033	0.0
Evapotranspiration	0.0	8,108
Total	56,840	56,838

Table 7. Water Budget for the Steady-State Simulation (afy)

selected wells for the time period where reliable historical drawdown measurements are available. Reliable water level data exists from 1950 to 1985 for most wells with the exception of few wells where water level data are available for different time periods.

The difference between observed and simulated drawdowns ranged from a few feet in some wells to more than 20 feet in others. However, the simulated rate of drawdown in most wells is similar to the observed rate. The differences in drawdowns is attributed to several factors the most important being the high uncertainty associated with the simulated historical pumping rates, observed drawdowns are measured in production wells and not in observation wells, and simulated drawdowns represent cell averages for one square mile. The difference is also attributed to the assumption that simulated pumping rates and recharge rates are constant for each stress period and for the entire simulation.

Figure 19 shows the remaining saturated thickness at the end of the transient simulation period (1990). In New Mexico, the remaining saturated thickness ranges from zero in areas near Portales and Clovis where total aquifer de-watering occurred to more than 100 feet east of Portales near the state line. Simulated saturated thickness ranged from less than 50 feet north, east, and west of Clovis to less than 100 feet south of the city in Roosevelt County. In Portales Valley, most areas around the city of Portales are de-watered. The simulated 1990 saturated thickness east of Portales ranged from less than 50 feet to more than 100 feet near the New Mexico-Texas state line. Greater remaining saturated thickness are simulated in Parmer County Texas (between 50 and 200 feet) whereas, the remaining saturated thickness in most of Bailey County, Texas, is less than a 100 feet with de-watered areas west of Muleshoe. De-watering resulted from the high withdrawal rates and limited saturated thickness simulated in the transient model.

The water budget for the transient simulation is shown in Table 8. Most of the artificial discharge by pumping comes from aquifer storage. Natural discharge by evapotranspiration decreased as the depth of the water table increased due to pumping. Recharge rate in the transient model are slightly less than recharge rate in the steady-state model due to an increase dry cells resulting from extensive pumping.

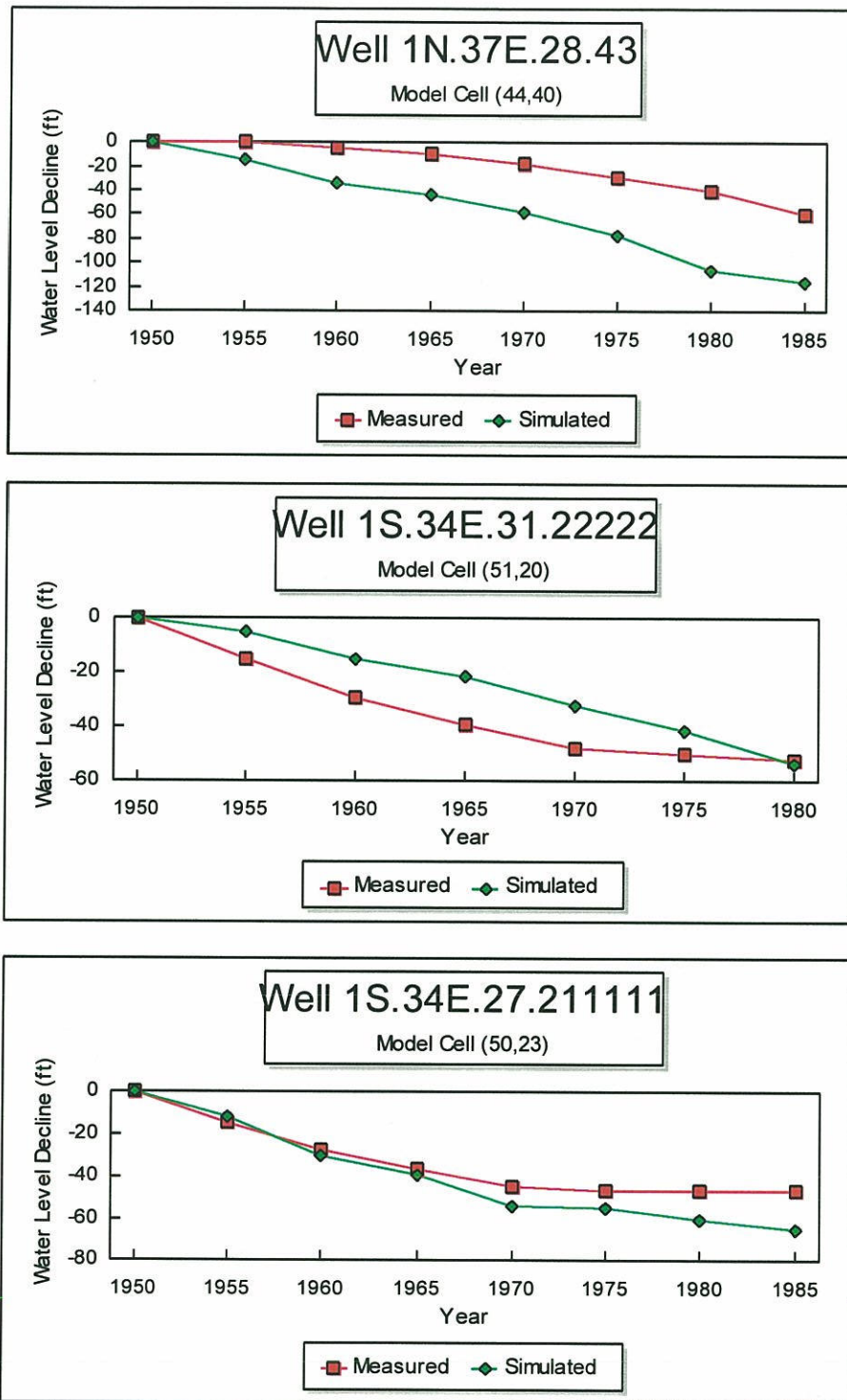


Figure 18. Simulated versus Observed drawdowns in Selected Wells.

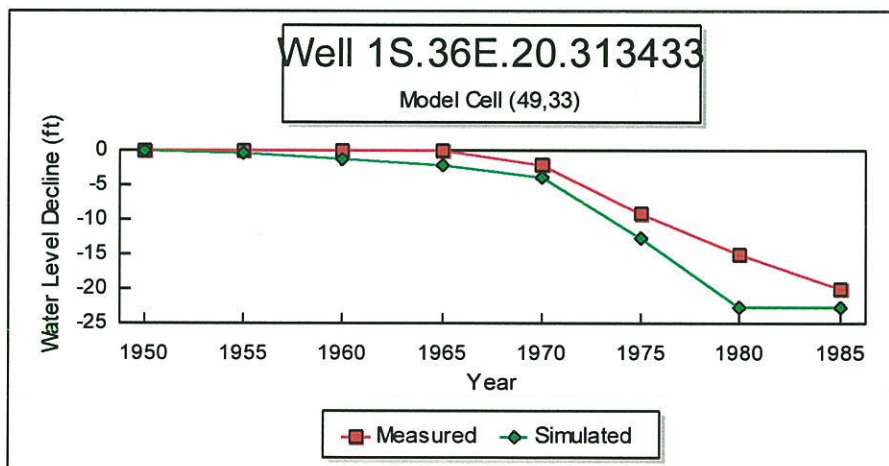
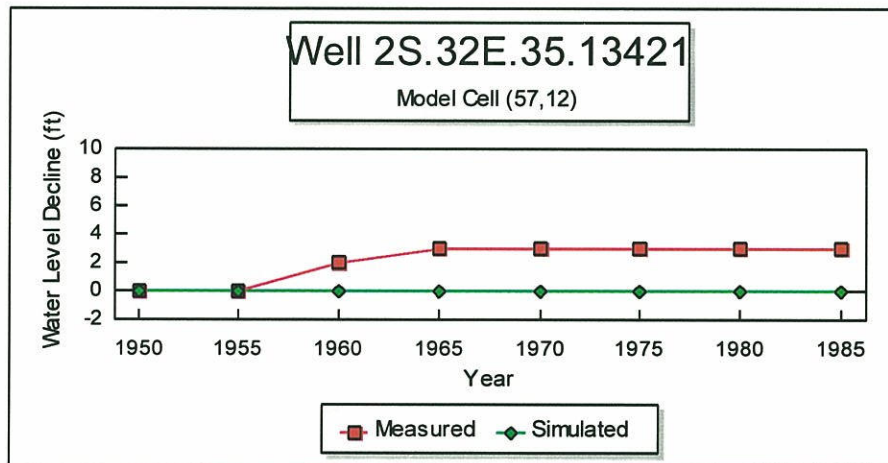
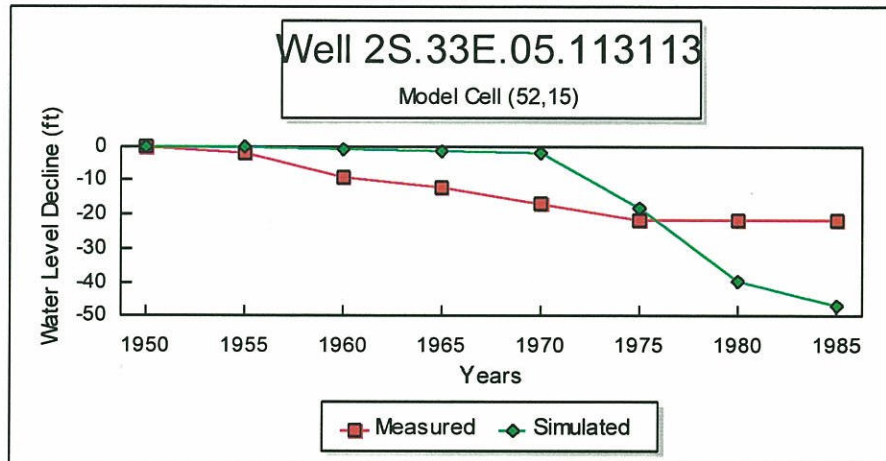


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

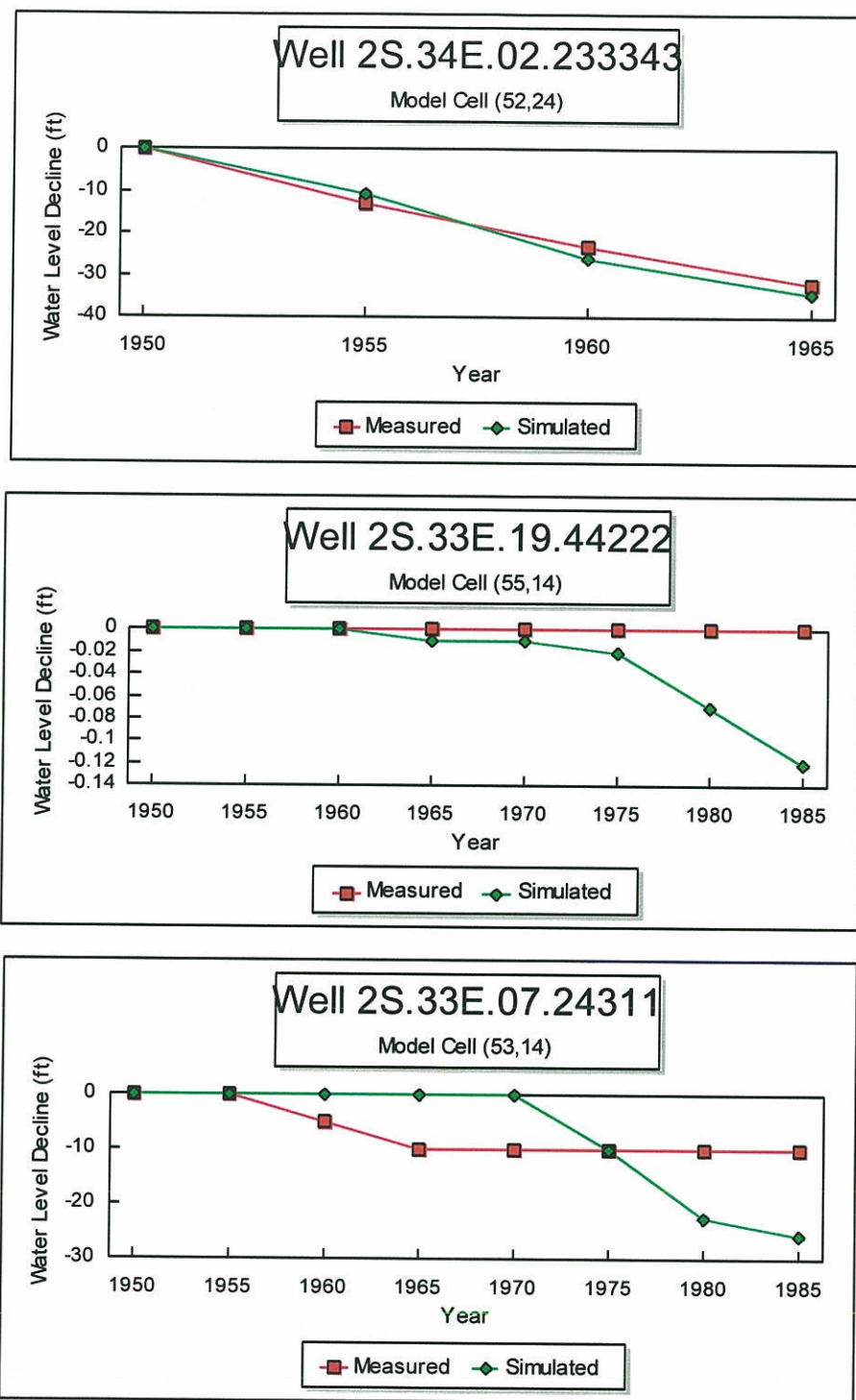


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

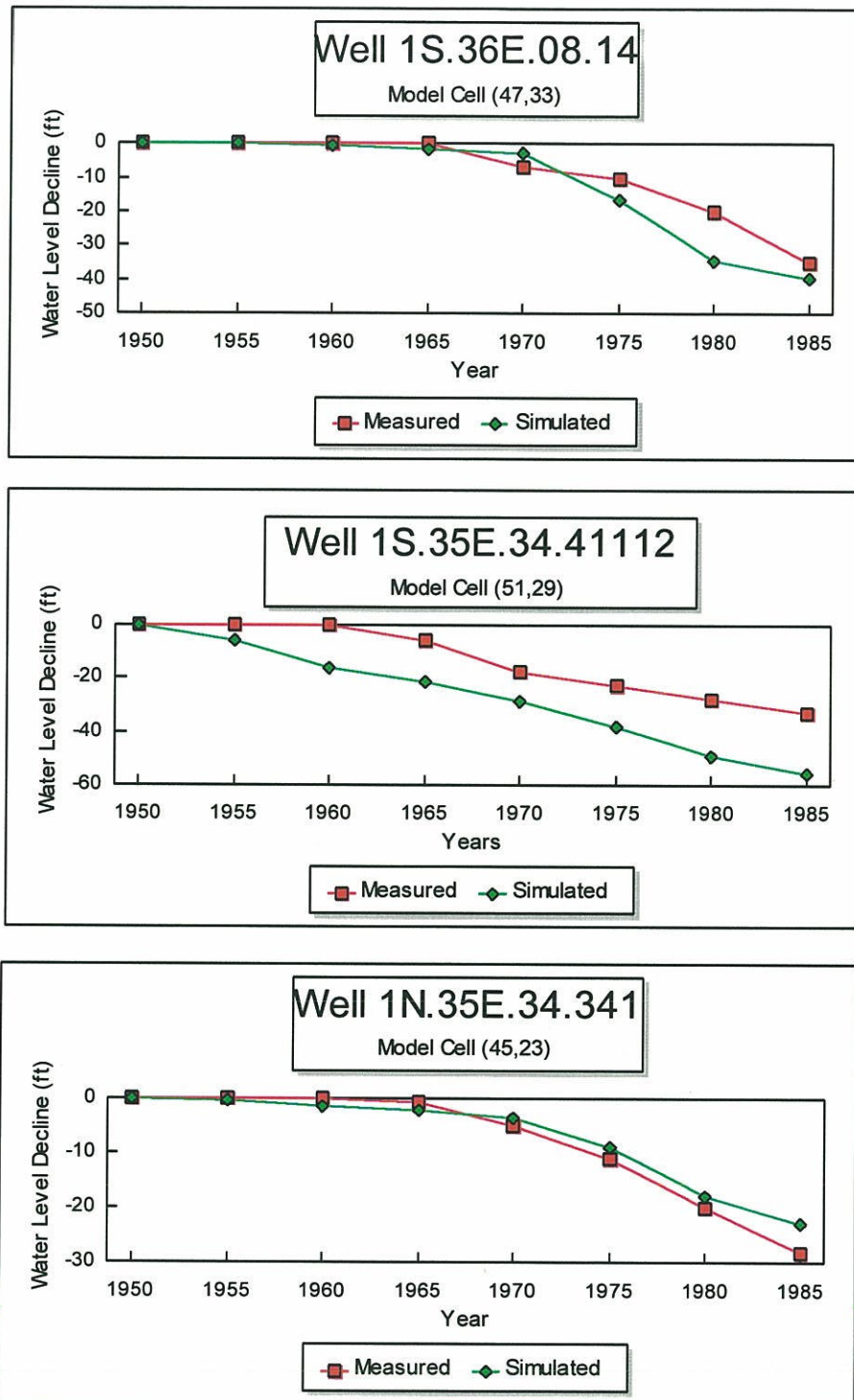


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

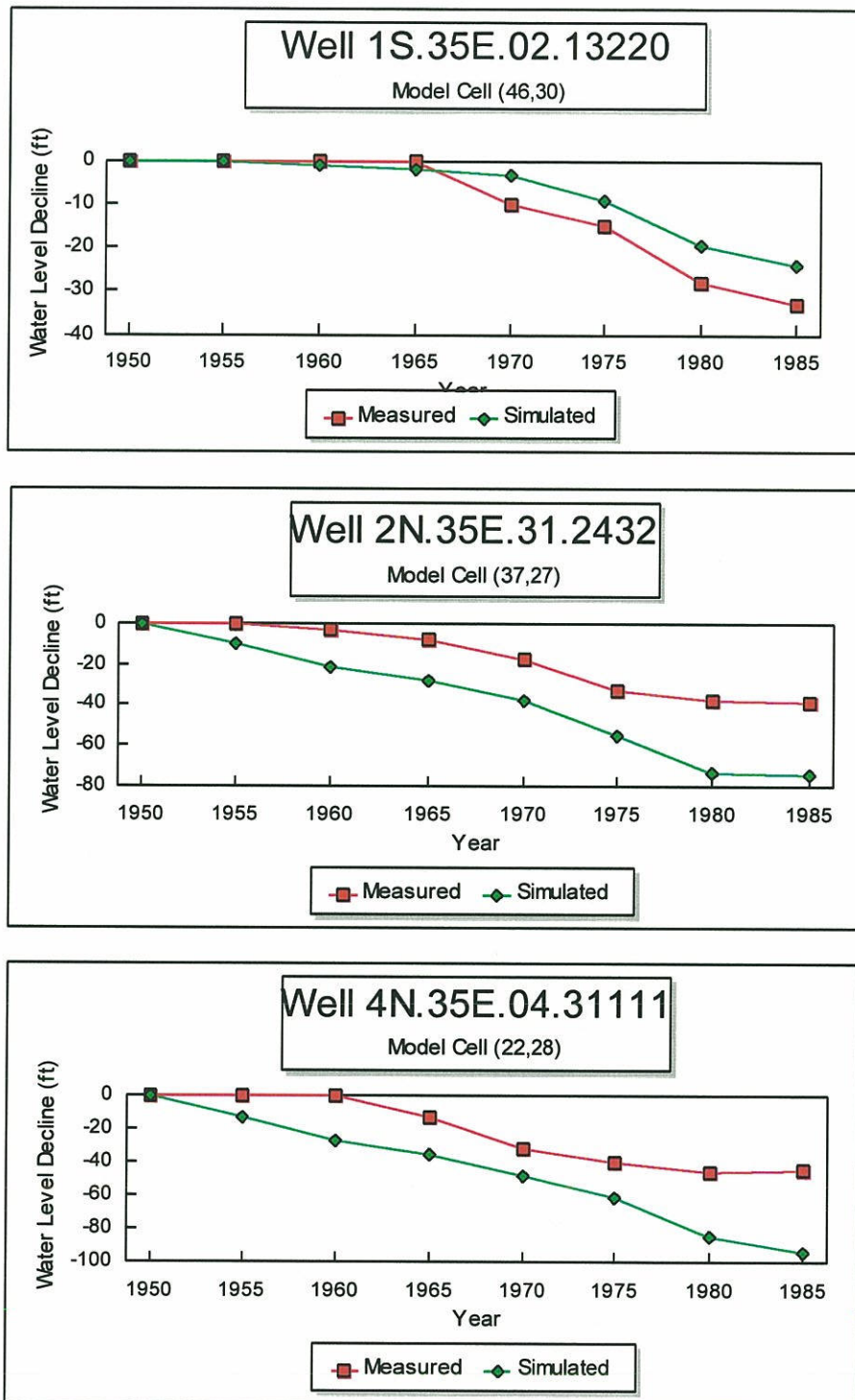


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

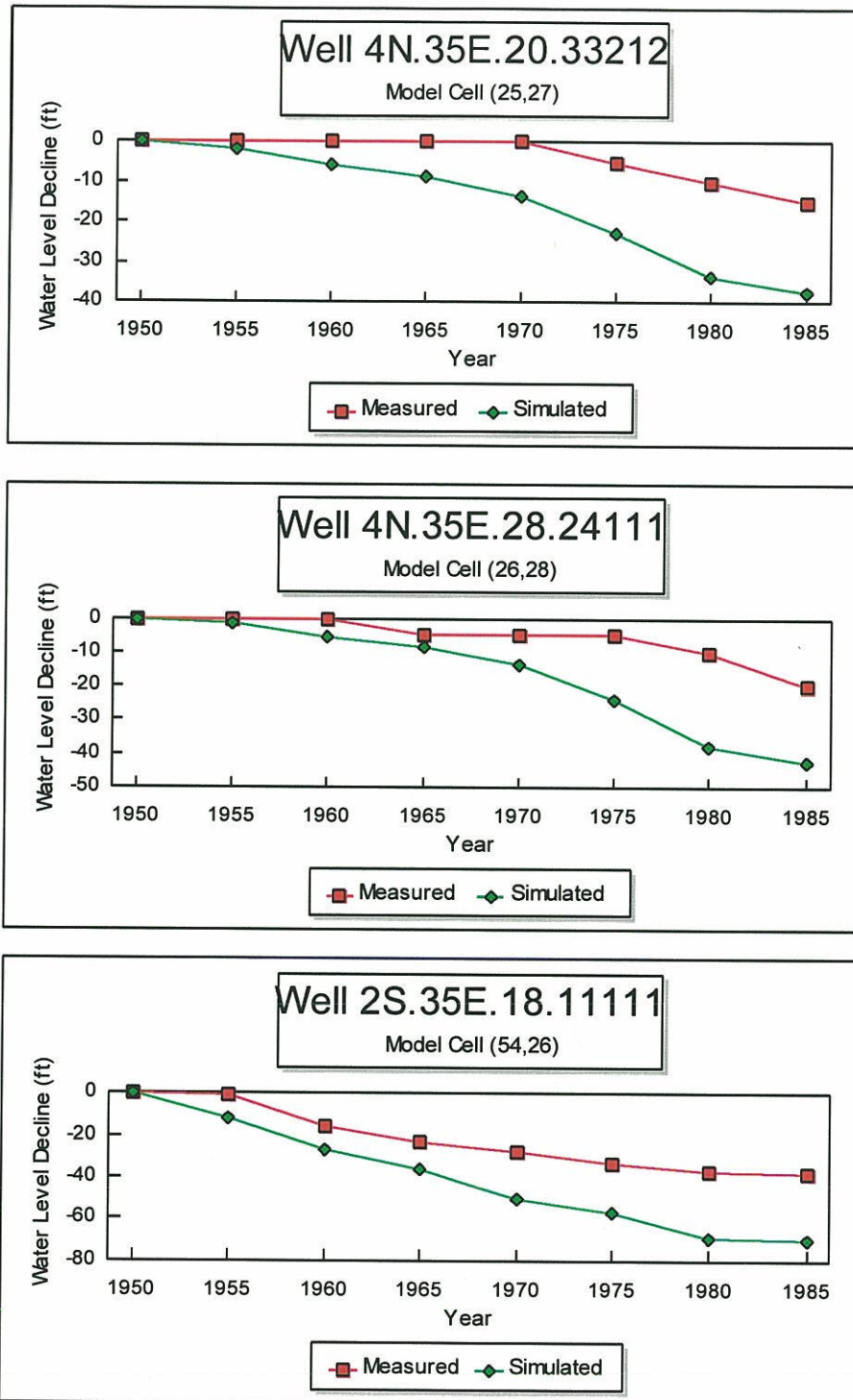


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

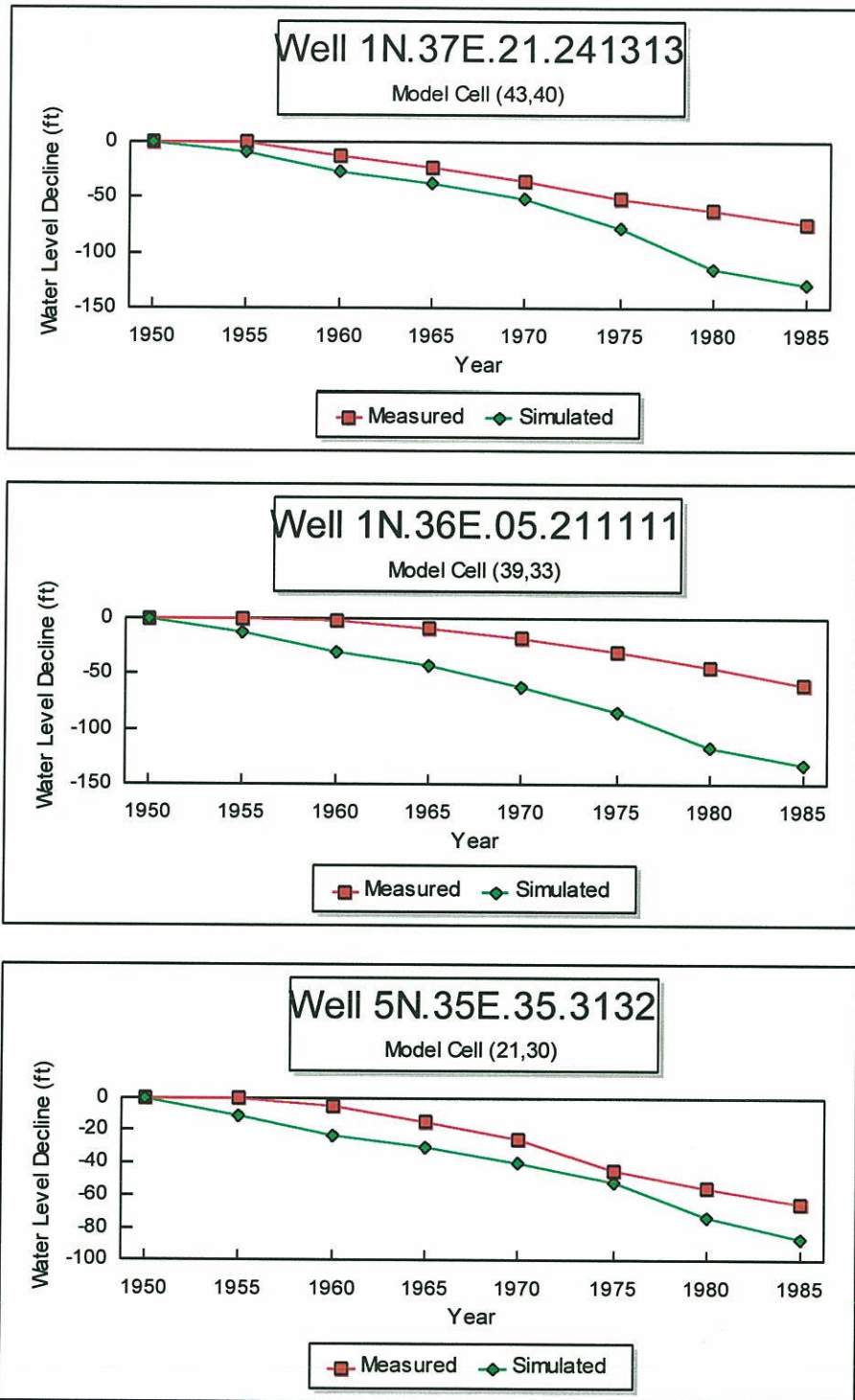


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

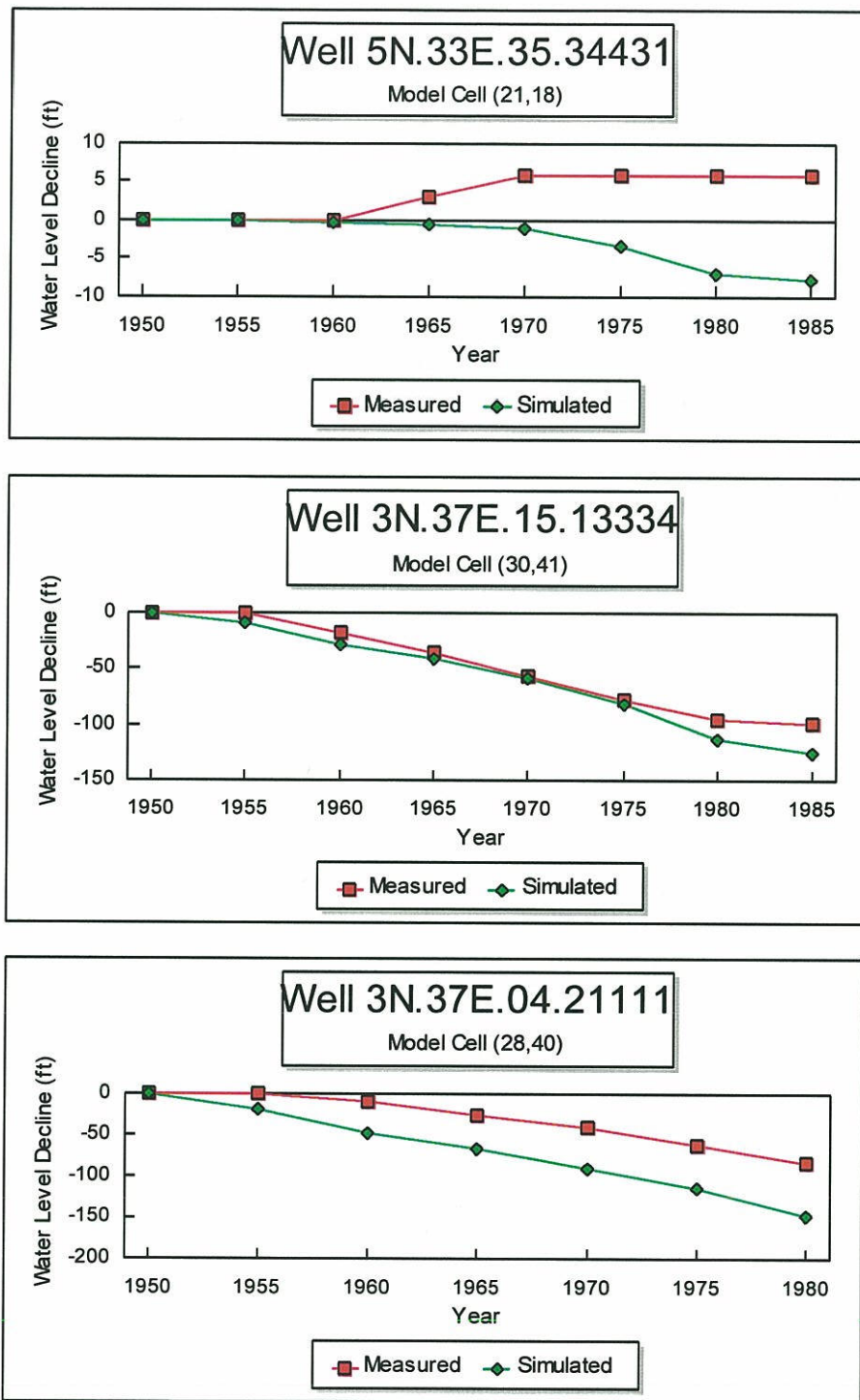


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

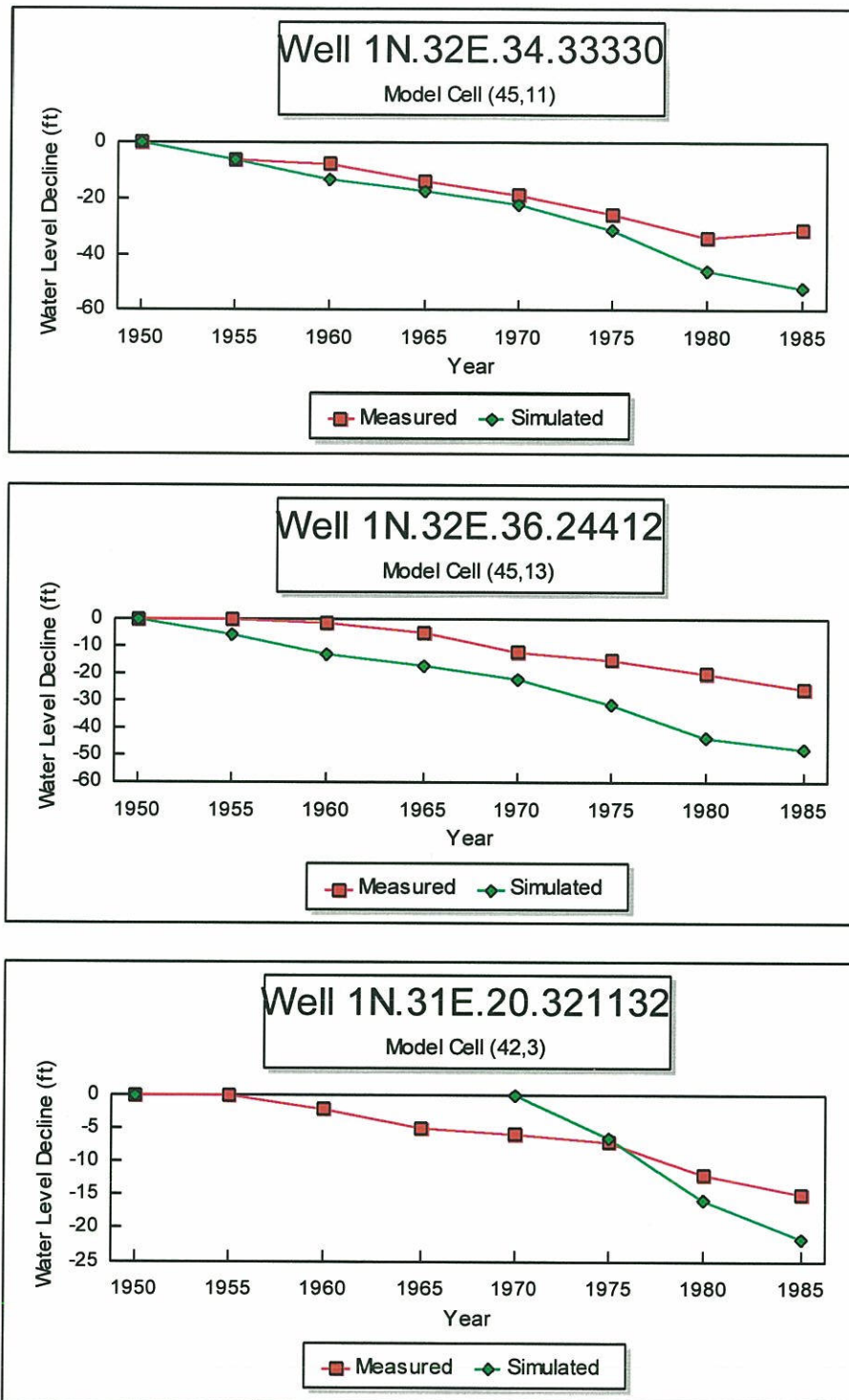


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

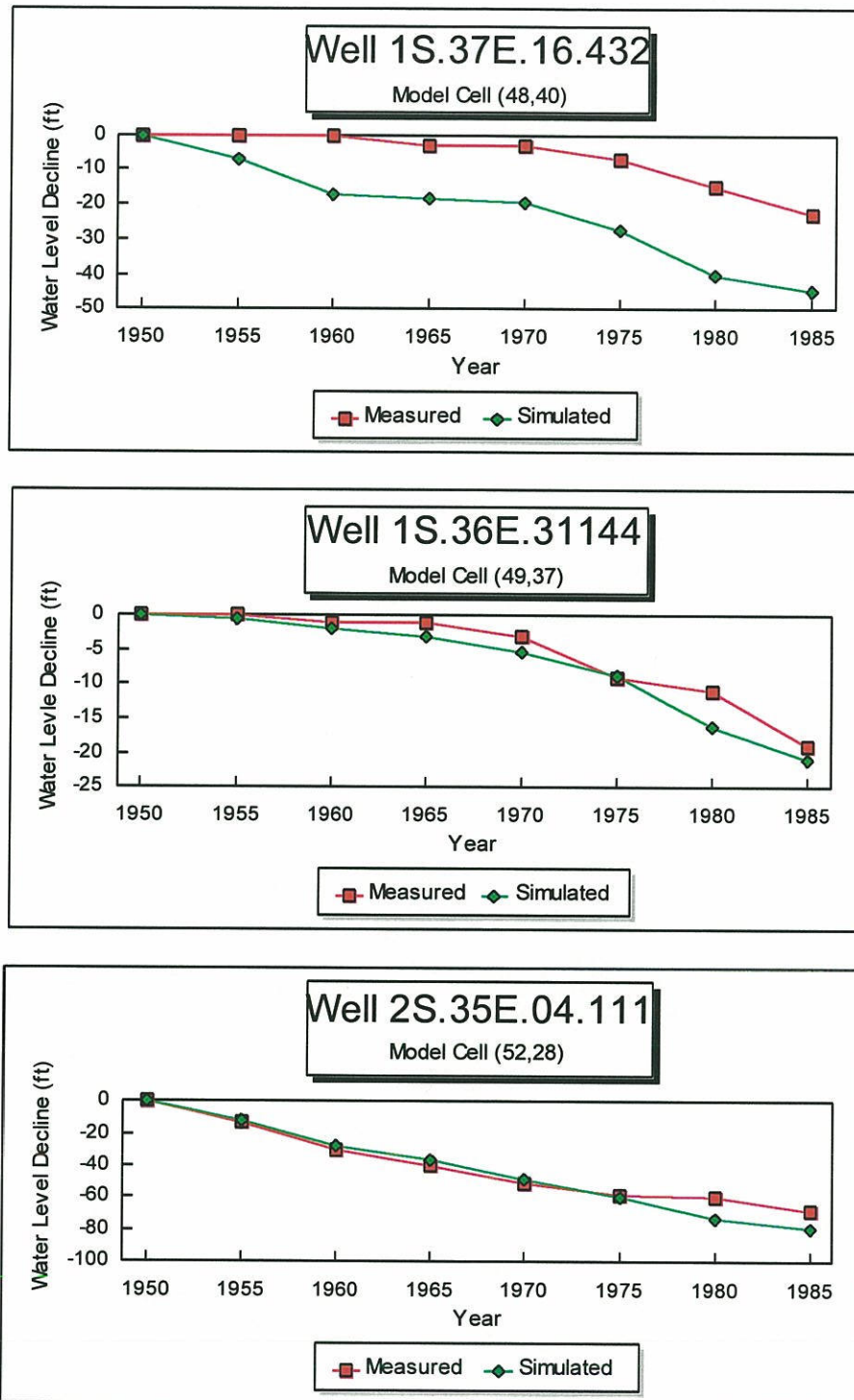


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).

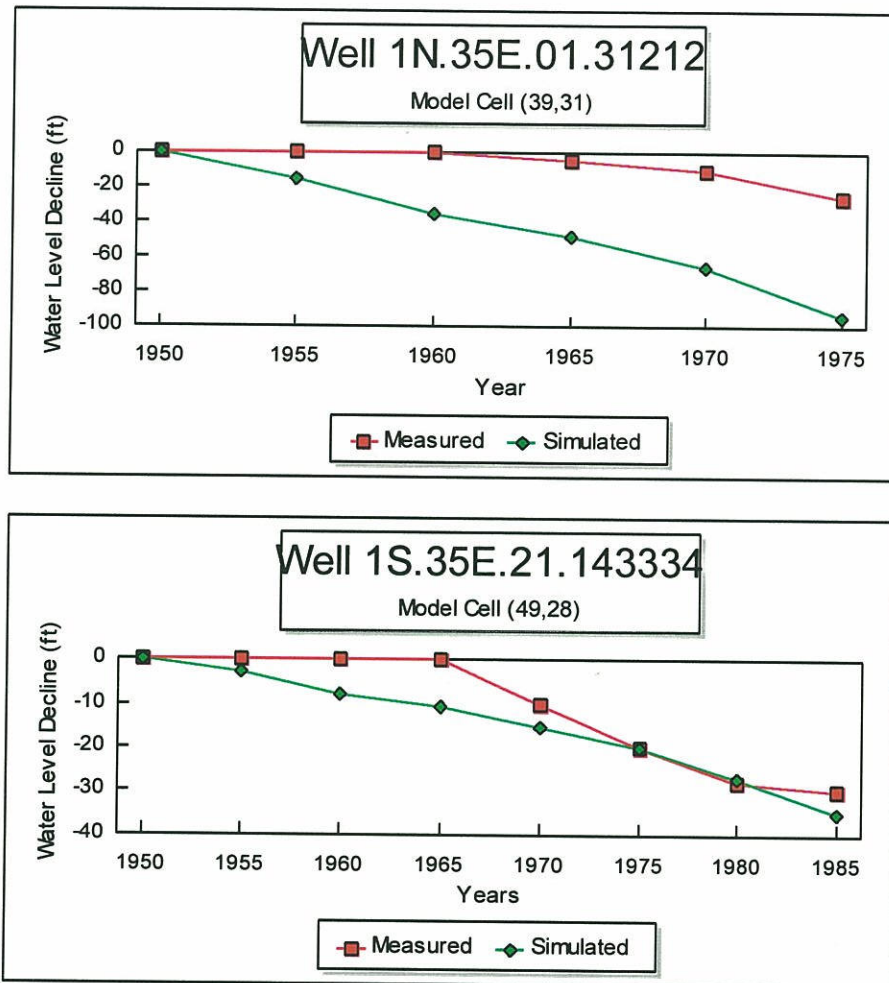
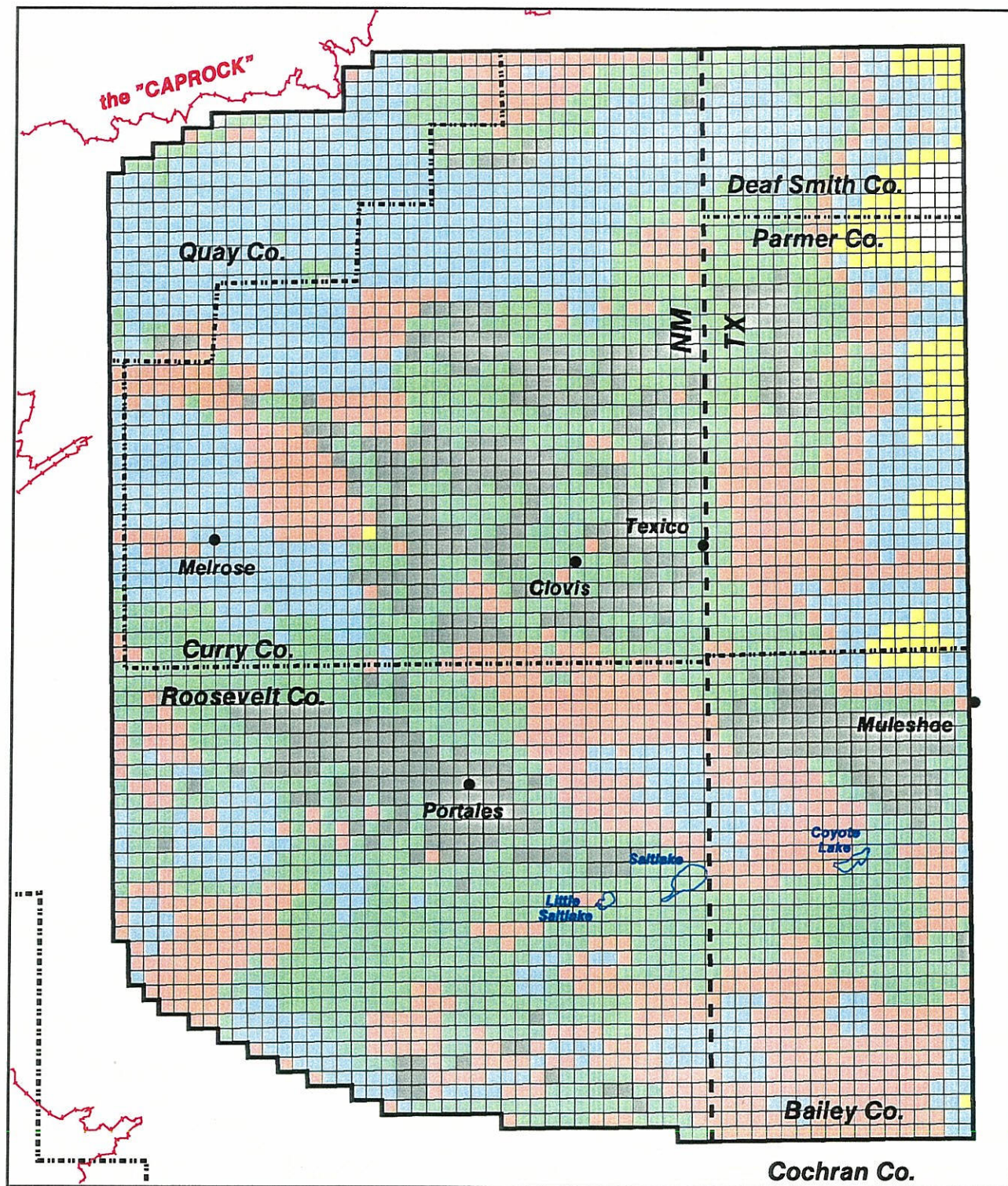


Figure 18. Simulated versus Observed drawdowns in Selected Wells (Cont.).



Legend

0	100-149
1-49	150-199
50-99	200-299

- Model Boundary
- State Line
- County Line

Figure 19: Remaining Saturated Thickness at the end of the 1990 Simulation Period (ft)

Source	Inflow to the Aquifer	Outflow from the Aquifer
Storage ¹	280,910	
Pumping wells and Specified Flows	11,751	335,106
Areal Recharge	43,512	0.0
Evapotranspiration	0.0	1,066
Total	336,173	3,366,172

1 Inflow to the aquifer from storage means the net amount of water removed from storage.

Table 8. Water Budget for the Transient Simulation at the end of the 1990 Simulation Period (afy).

C. Groundwater Flow Across the New Mexico-Texas State Line.

Groundwater flow across the New Mexico-Texas state line was computed to determine the effects of Texas pumping on groundwater flow from New Mexico into Texas (Figure 20). State-line flows were calculated for steady-state conditions and for each stress period of the transient model. The results show that pumping in Texas caused an initial increase in

groundwater flow across the state line. As pumping continued on both sides of the state line, the rate of groundwater flow across the entire Curry-Portales section of the state line decreased to less than pre-development level. However, in the vicinity of Clovis, flow across the state line increased initially and then decreased to -48 afy in 1990. Negative flow indicates that in the vicinity of Clovis pumping induced flow from Texas to New Mexico. In the vicinity of Portales Valley, flow across the state line decreased to less than pre-development level but did not change direction (Figure 21).

To examine the effect of Texas pumping on groundwater levels in the Curry-Portales area, New Mexico, a simulation was performed with pumping in Texas only. Likewise, a simulation was also performed to determine the effect of New Mexico pumping on water levels in New Mexico and Texas. Figures 22 and 23 shows the 1909-1990 drawdowns in New Mexico resulting from New Mexico and Texas pumping, respectively. Texas pumping induced 50 feet of drawdown in New Mexico along the New Mexico-Texas state line. Drawdown decreased as we moved away from the state line into New Mexico. In the vicinity of Clovis, 5 feet of drawdown was induced by Texas pumping while less than 2 feet drawdown was induced in the vicinity of Portales Valley. Pumping in New Mexico also induced drawdown in Texas. Greatest drawdowns are observed along the state line (approximately 50 feet) and decreased as we moved east into Texas. Thus, it is apparent that pumping in one state has a direct effect on water levels in the other and the greatest effects are near the New Mexico-Texas state line.

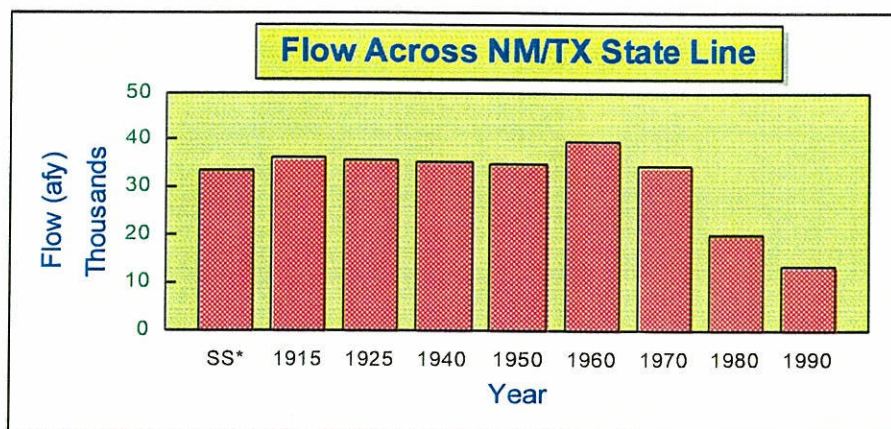


Figure 20. Flow Across the State Line Under Steady-State and Transient Conditions.
* Indicate Flow Across the State Line Under Steady-State Conditions.

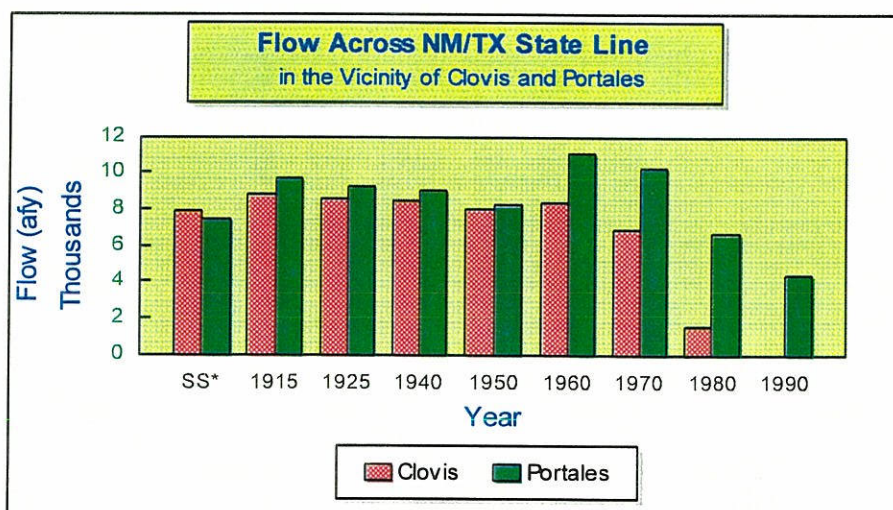
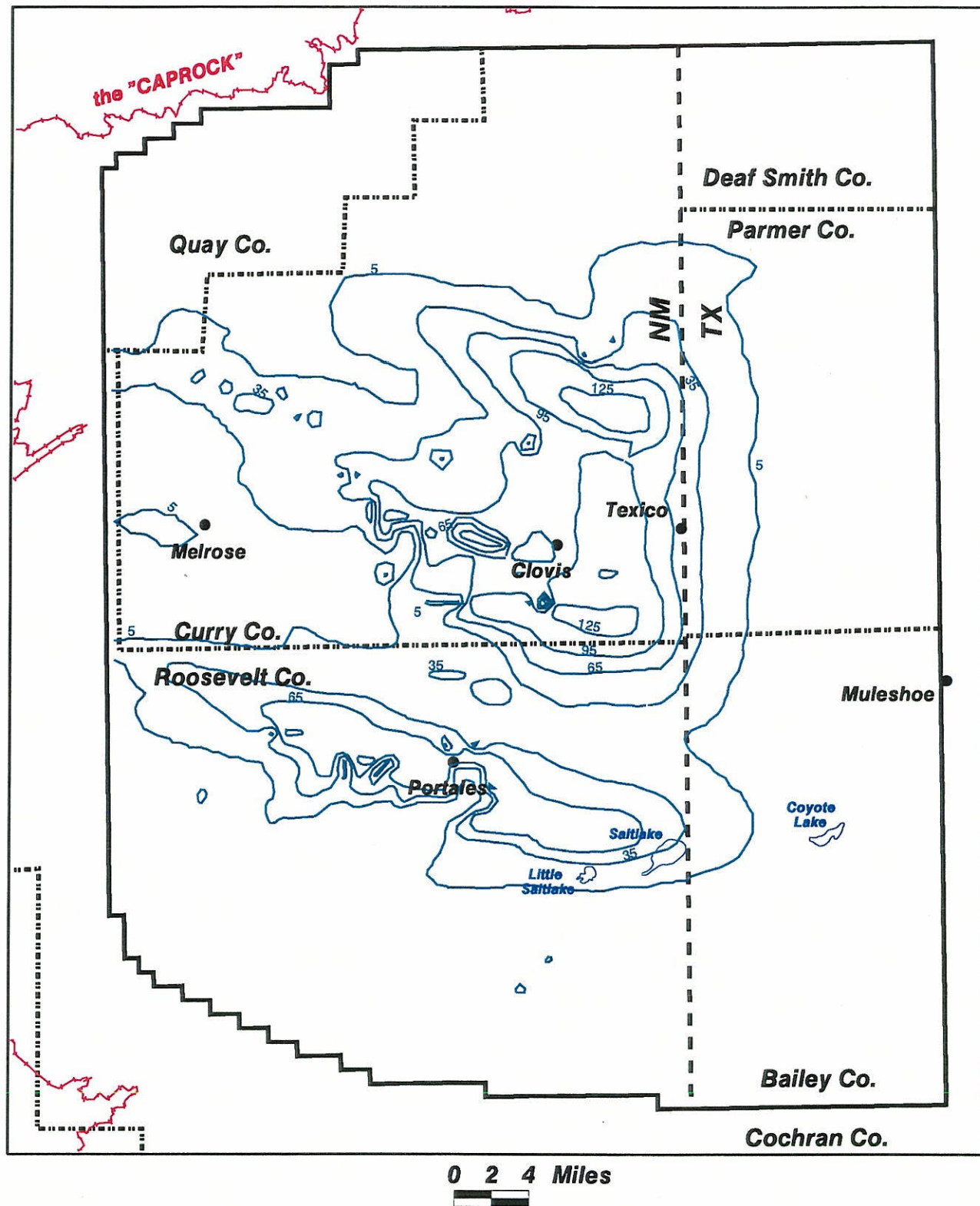


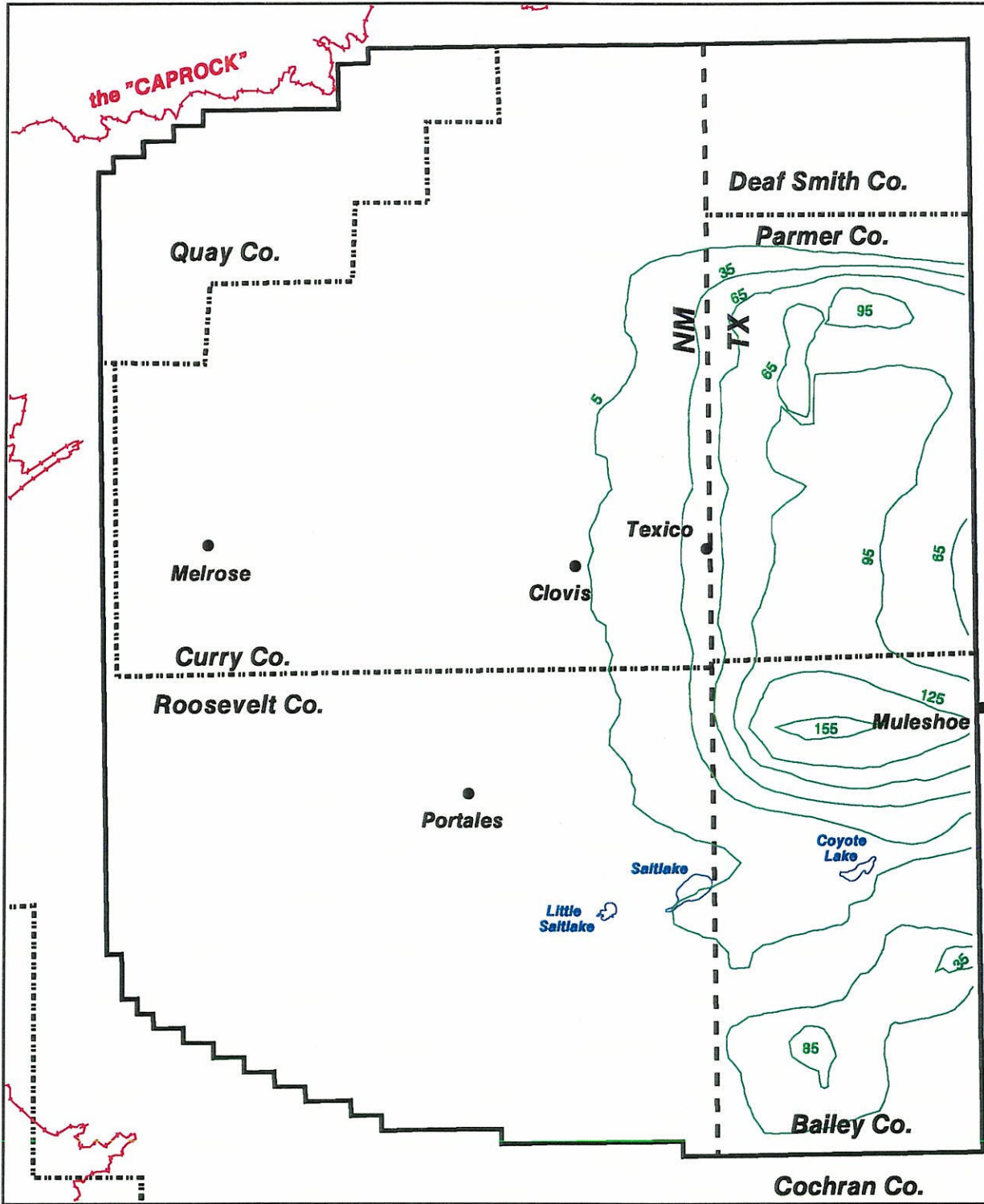
Figure 21. Flow Across the State Line in the Vicinity of Clovis and Portales.
* Indicate Flow Across the State Line Under Steady-State Conditions.



Legend

- Model Boundary
- State Line
- County Line
- Water-Table Decline

**Figure 22: Simulated Drawdown
Due to New Mexico Pumping (ft)
(1909-1990)**



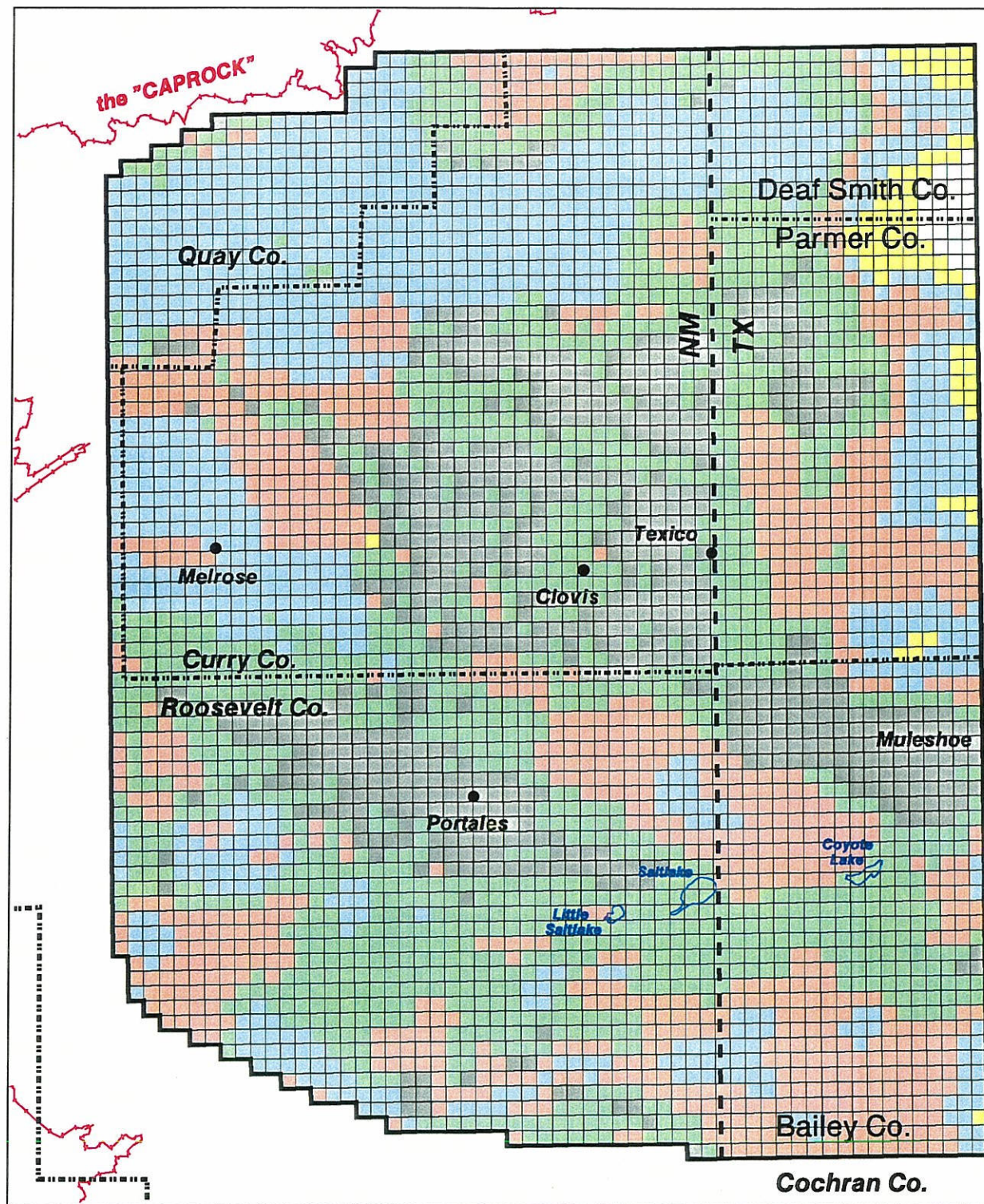
- Legend**
- Model Boundary
 - State Line
 - County Line
 - Water-Table Decline

**Figure 23: Simulated Drawdown
Due to Texas Pumping (ft)
(1909-1990)**

V. PROJECTED GROUNDWATER PRODUCTION

Projected groundwater withdrawals for the year 2040 were simulated using the estimated 1981-1990 pumping rates. Pumping rates from the last stress period were chosen because they represent the long term average (1926-1990) of 494,000 afy. Simulations through the year 2040 indicate that water level declines in the basin will continue with total de-watering of areas around Clovis and Portales Valley in New Mexico and north of Bailey County in Texas. Actually, concentrated pumping in Curry and Roosevelt Counties in New Mexico as well as Bailey County in Texas will de-water large portions of the most productive areas of the basin as early as the year 2010. Parts of the basin where the remaining 1990 saturated thickness is less than 50 feet will not sustain continued pumping beyond the year 2000 or 2010. The greatest simulated drawdowns in the year 2040 are along the state line due to the combined effect of pumping in New Mexico and Texas. In these areas the remaining saturated thickness decreased to as low as 5 feet (Figures 24 to 27).

Table 9 shows water budgets for the years 2000, 2010, 2020, and 2040. Aquifer storage contributes about 80% of the extracted water from the basin (except for the year 2040 where aquifer storage contributes only 70% of the extracted water) causing an average water level decline of approximately 2 to 5 feet/year. Areal recharge and subsurface flow contribute only about 20% of the extracted water from the basin. Current depletion rates can not be sustained indefinitely and will eventually de-water the basin. It should be noted that these projections are based on the present irrigation of approximately 100,000 acres in New Mexico. According to OSE record there are approximately 127,000 acres with permitted irrigation rights (OSE 1991 annual report of basin administration). The annual rate of water level decline will increase if additional permitted acreages are brought back into irrigation.

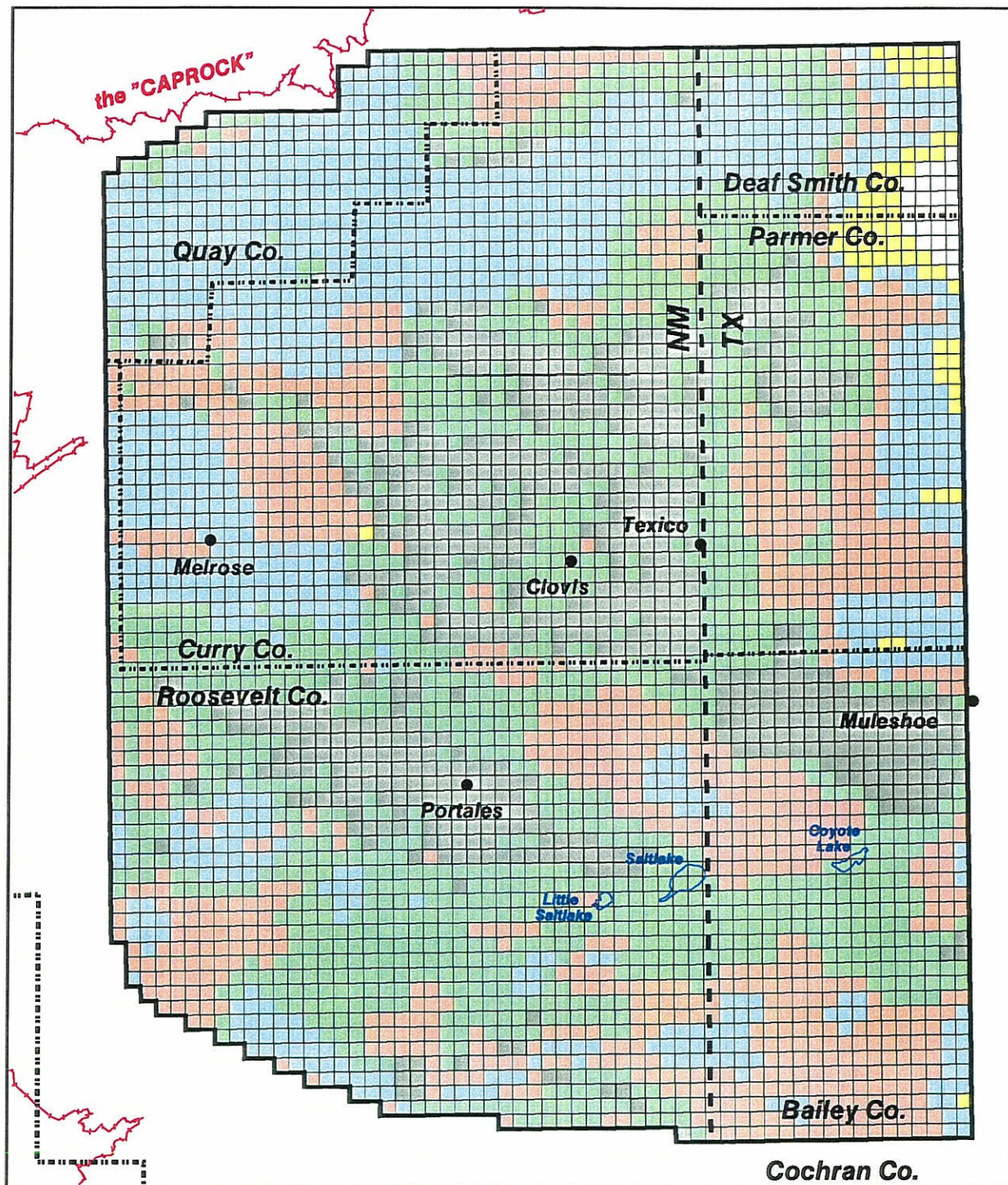


Legend

0	100-149
1-49	150-199
50-99	200-299

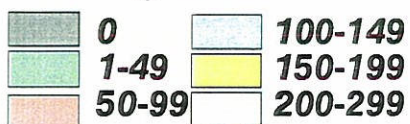
- Model Boundary
- State Line
- County Line

**Figure 24: Projected
Remaining Saturated
Thickness in the Year 2000 (ft)**



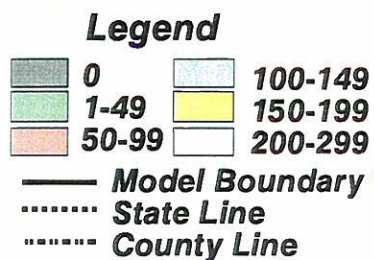
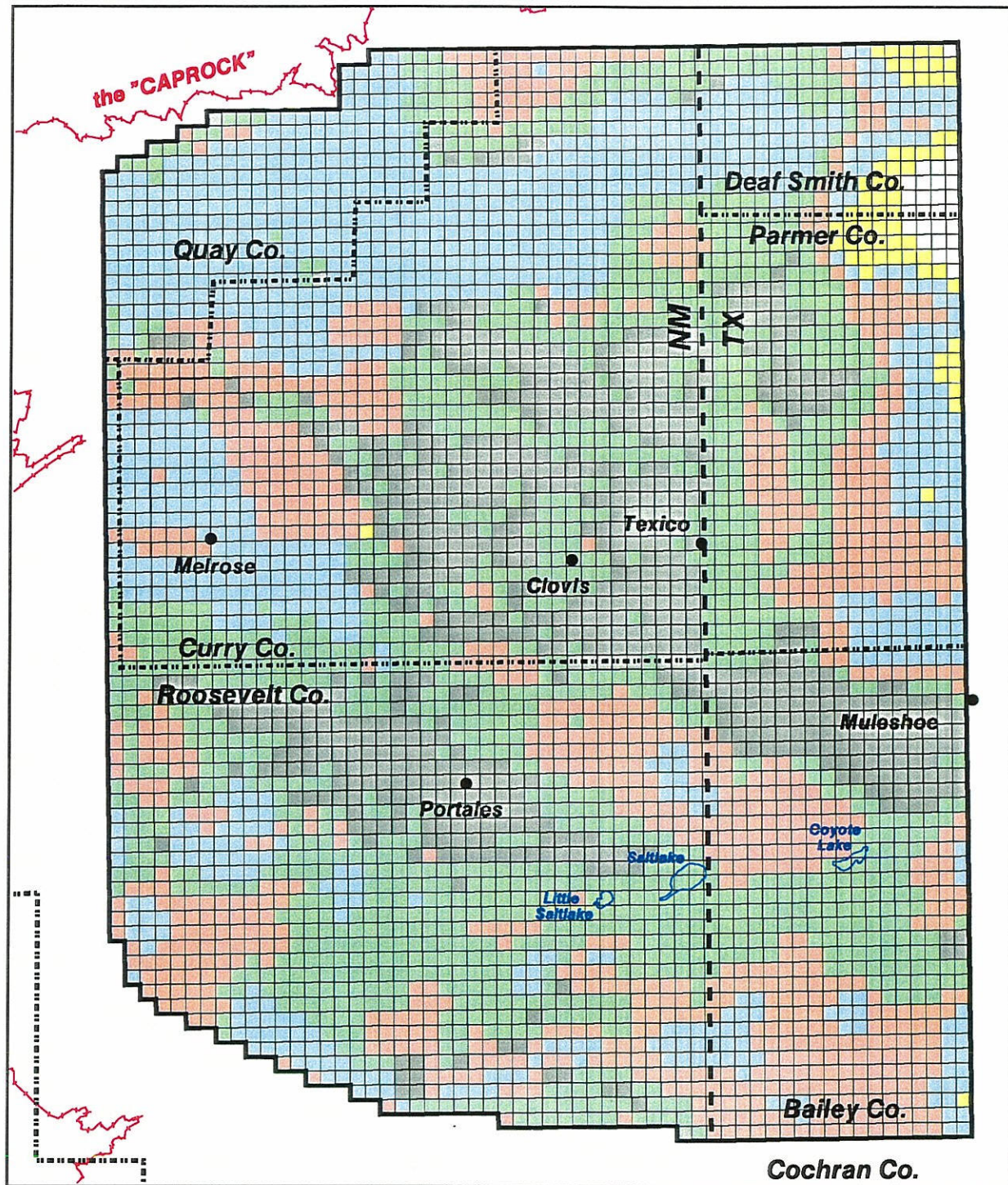
0 2 4 Miles

Legend

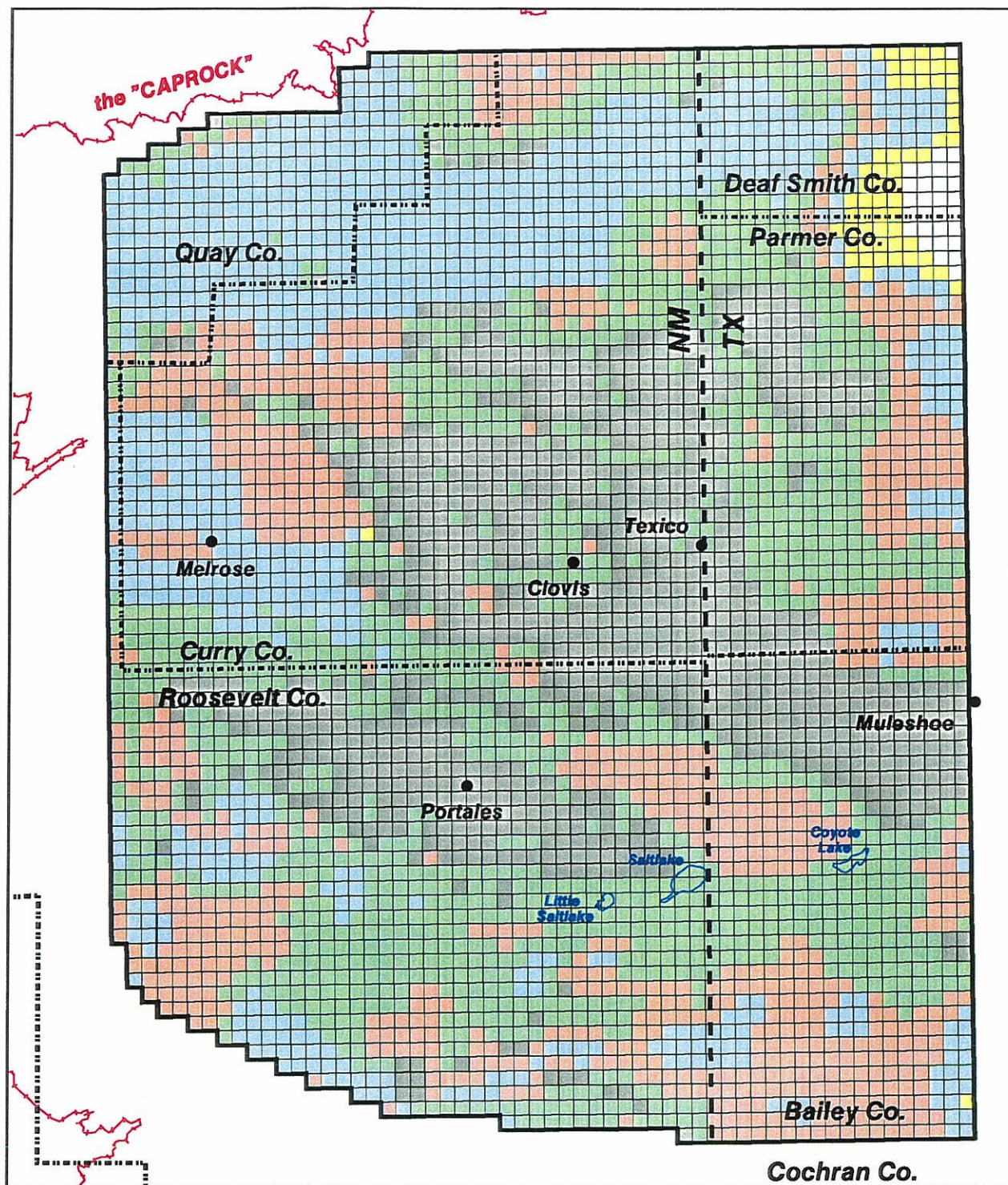


- Model Boundary
- State Line
- County Line

**Figure 25: Projected
Remaining Saturated
Thickness in the Year 2010 (ft)**



**Figure 26: Projected
Remaining Saturated
Thickness in the Year 2020 (ft)**



Legend

0	100-149
1-49	150-199
50-99	200-299

- Model Boundary
- State Line
- County Line

**Figure 27: Projected
Remaining Saturated
Thickness in the Year 2040 (ft)**

Year	2000	2010	2020	2040
Inflow to the Aquifer				
Storage ¹	207,757	204,710	191,088	113,137
Specified Flows	11,710	11,710	11,710	11,710
Areal Recharge	41,238	41,120	40,622	37,116
Evapotranspiration	0.0	0.0	0.0	0.0
Total	260,705	257,540	243,420	161,963
Outflow From the Aquifer				
Pumping wells and				
Specified Flows	259,767	256,698	242,519	161,850
Areal Recharge	0.0	0.0	0.0	0.0
Evapotranspiration	937	926	904	783
Total	260,709	257,619	243,419	161,963
1 Inflow to the aquifer from storage means the net amount of water removed from storage.				

Table 9. Water Budget for Projected Withdrawals.

VI. SENSITIVITY ANALYSIS

A model's ability to predict the response of the aquifer system to applied stresses depends on how well the actual physical system is mathematically represented in the model. Major assumptions and generalizations are made to characterize the physical system. These assumptions include selecting the average and areal distribution of hydraulic conductivity, recharge, evapotranspiration, and specific yield. These characteristics are not known to any degree of certainty, and the system responds differently to variations in any of these parameters. To quantify the effect of these characteristics on model results a sensitivity analysis was performed by varying the values of these parameters within a specified range.

The response of the system to changes in a parameter is tested by varying that parameter while keeping others constant. Steady-state simulations were performed to predict the system response to changes in recharge and hydraulic conductivity. Transient simulations were performed to predict the system response to changes in specific yield.

A. Steady-State Simulations.

1. Hydraulic Conductivity.

The effect of increasing and decreasing hydraulic conductivity by 5%, 15%, and 20% on simulated steady-state water levels is shown in Table 10. Shown are the mean error (ME), mean square error (MSE), and the root mean square error (RMSE) for the differences between simulated and observed steady-state water levels for New Mexico, as a result of changes in hydraulic conductivity. Increasing or decreasing hydraulic conductivity by 5% had little effect on the difference between observed and simulated water levels. Increasing and decreasing hydraulic conductivity by 15% and 20% increased the MSE and the RMSE indicating that the difference between simulated and observed water levels increased.

2. Recharge.

The effect of increasing and decreasing recharge by 5%, 15%, and 20% on simulated steady-state water levels is shown in Table 11. Decreasing and increasing recharge increased the difference between simulated and observed water levels. Decreasing recharge increases the difference between simulated and observed water levels more than increasing recharge. Some cells with small saturated thicknesses dried out when recharge was reduced by 20% indicating that simulated water levels are more sensitive to decreases than to increases in recharge rates.

	Standard	Decreasing K			Increasing K		
		5%	15%	20%	5%	15%	20%
ME	1.7	-0.7	-6.8	-10.2	3.9	8.4	10.7
MSE	7.7	7.4	10.4	13.0	8.8	12.2	14.4
RMSE	10.6	10.0	12.1	14.6	11.2	14.3	16.8

Table 10. Statistical Comparison Between Standard and Varied Hydraulic Conductivity (K) in the Model.

	Standard	Decreasing R			Increasing R		
		5%	15%	20%	5%	15%	20%
ME	1.7	4.4	10.6	15.4	-1.0	-6.6	-9.1
MSE	7.7	9.0	14.0	17.9	7.4	10.0	11.8
RMSE	10.6	11.3	16.4	20.7	10.0	11.8	13.4

Table 11. Statistical Comparison Between Standard and Varied Recharge (R) in the Model.

B. Transient Simulations.

1. Specific Yield.

The sensitivity of the model to changes in specific yield values is tested using 10% and 15% increase and decrease in specific yield. Sensitivity to specific yield is tested only for the transient model. Figure 28 shows the change in simulated water levels as a result of changes in specific yield for selected wells. Increasing specific yield decreased drawdowns while decreasing specific yield increased drawdowns. Drawdowns changed by more than 10 feet (10 to 15 feet) when specific yield was changed by 15%. Simulated water levels seem to be most sensitive to specific yield.

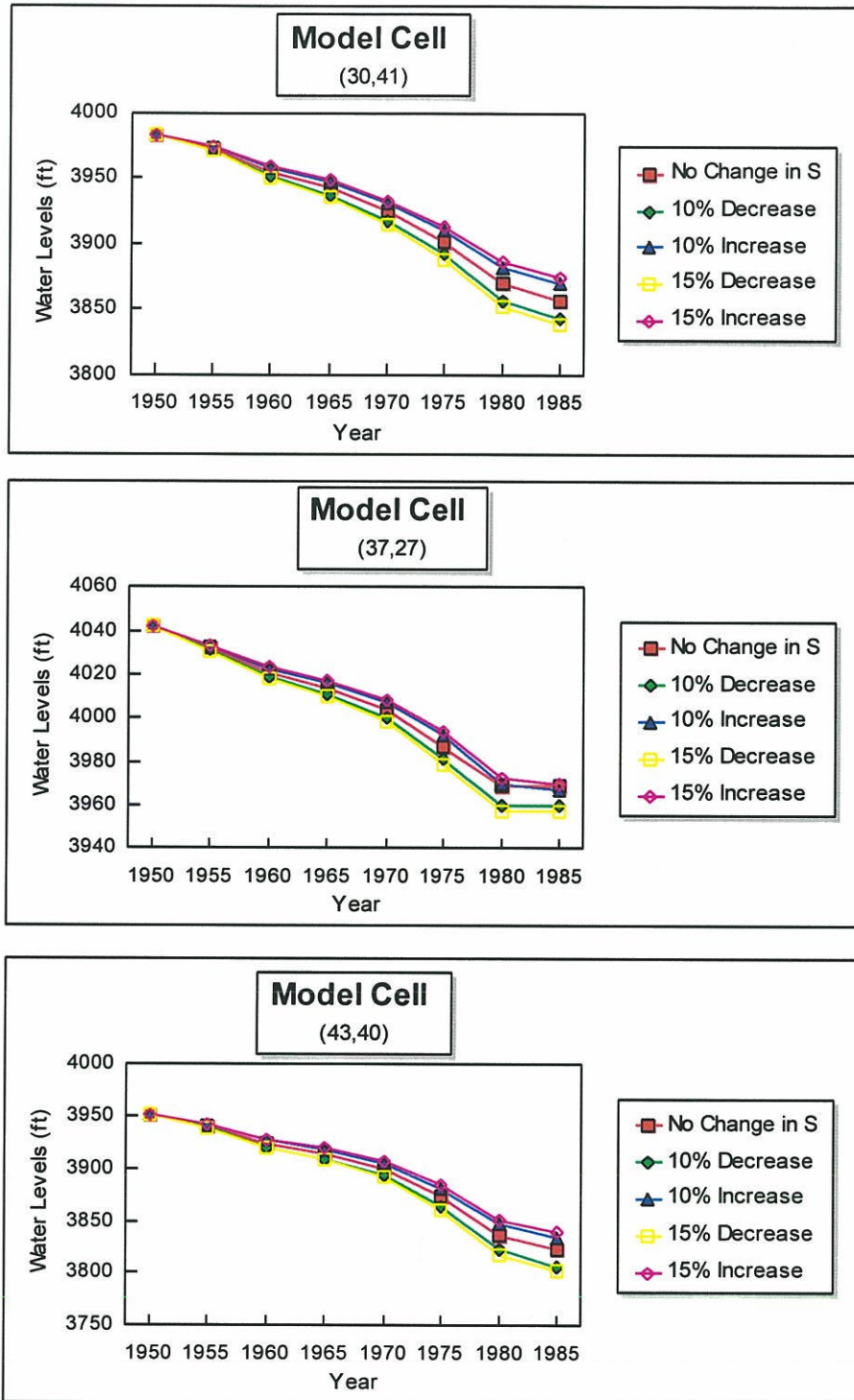


Figure 28. Effects of Changes in Specific Yield on Simulated Water Levels in Selected Wells.

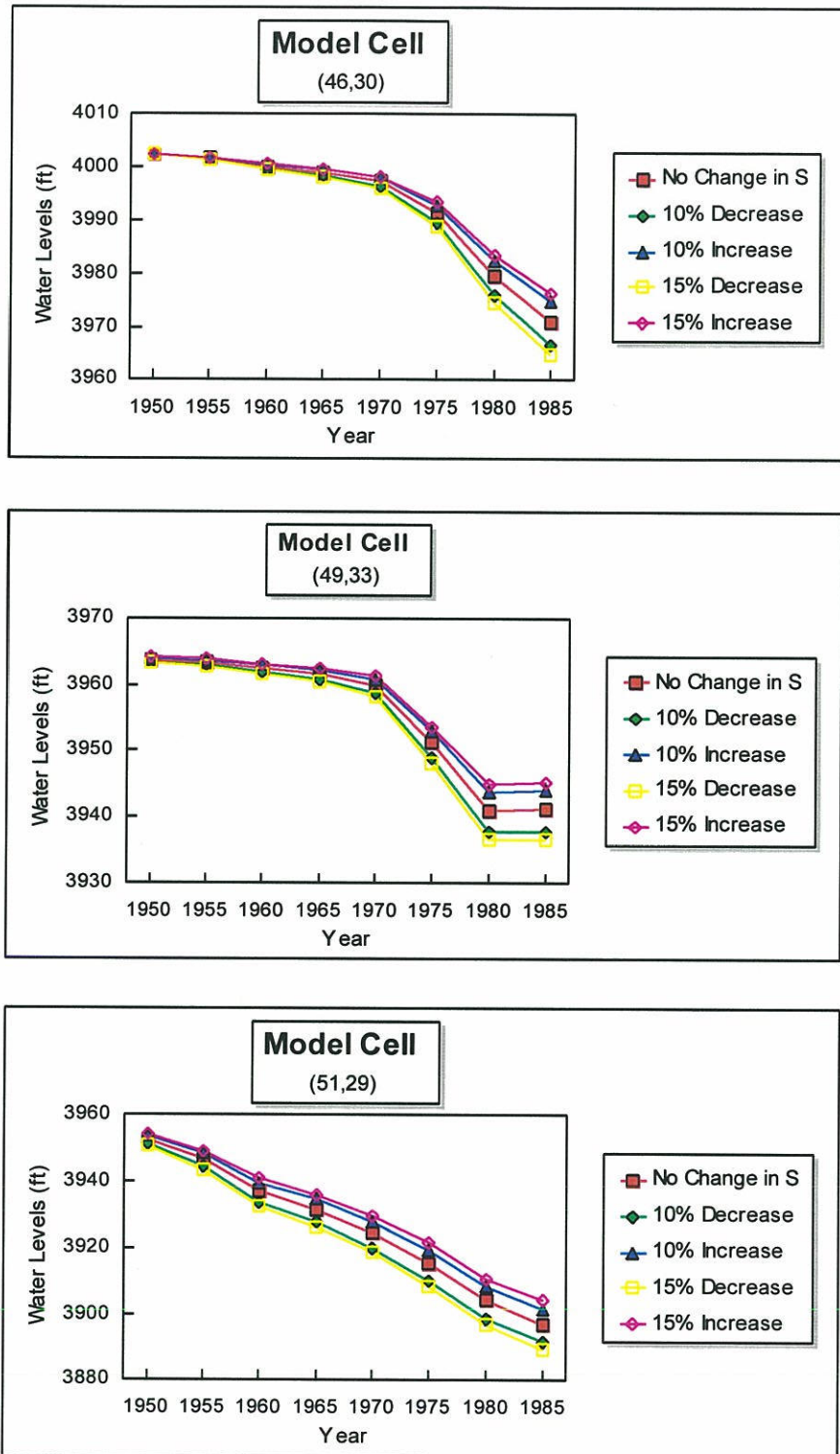


Figure 28. Effects of Changes in Specific Yield on Simulated Water Levels in Selected Wells. (Cont.)

VII. SUMMARY AND CONCLUSION

A two dimensional, finite-difference groundwater flow model was developed for Curry County and Portales Valley groundwater basins. The developed model may be used to estimate the effect of future groundwater withdrawals on water levels. Aquifer parameters, hydraulic conductivity, areal recharge, evapotranspiration, and specific yield were estimated within a plausible range reported by previous investigators. Estimated parameters resulted in an acceptable match between simulated and observed water levels. Fifty three percent of simulated steady-state water levels were within 10 feet of the observed pre-development water levels. However, in the area of primary groundwater development in New Mexico, 71% of the simulated water levels are within 10 feet of the observed levels. The rate of simulated drawdowns in the transient model matched the rate of observed drawdowns in selected wells in the basin.

Sensitivity analyses were performed to study the effects of changes in estimated hydraulic conductivity, recharge rate, and specific yield on simulated water levels. Aquifer parameters tested were varied while others remained unchanged from the standard value. Model response to variations in hydraulic conductivity and recharge rates was tested for pre-developed conditions. Increasing or decreasing hydraulic conductivity and recharge rates increased the difference between observed and simulated steady-state water levels. The response of the aquifer to variation in specific yield was tested for transient conditions. Changes in specific yield caused simulated drawdowns in selected wells to change.

A projected simulation to the year 2040 was performed to determine if the current saturated thickness of the aquifer can sustain the 1981-1990 withdrawal rates for 40 years. The 1981-1990 withdrawal rates represent the long term average (1926-1990) pumping rates from the basin. The remaining 1990 saturated thickness in the basin vary from one place to another and ranges from zero feet in some areas where excessive historical pumping resulted in dry areas to more than 100 feet in some parts of New Mexico and Texas. Historical withdrawals de-watered large areas in Portales Valley and Curry County, New Mexico, as well as parts of Bailey County in Texas, particularly west of Muleshoe. Under current withdrawal rates, water level declines of 2.5 and 3 to 5 feet per year were simulated in Roosevelt and Curry Counties, respectively. Similar rates of water level declines (3-5 feet per year) were simulated in Texas. Thus, with the current remaining saturated thickness of less than 100 feet in most areas, and with the average simulated rate of decline, the Ogallala Formation will not sustain the current production rates to the year 2040. Parts of the basin where the remaining 1990 saturated thickness is less than 50 feet will not sustain continued pumping beyond the year 2000 or 2010. De-watered areas will increase to cover large portions of Curry and Roosevelt Counties in New Mexico and Bailey and Parmer Counties in Texas.

The effects of Texas pumping on New Mexico water levels were examined by performing a historical simulation with no pumping in New Mexico. Simulated drawdowns indicated that Texas pumping has induced 5 to 50 feet drawdown in New Mexico. The largest drawdowns are along the state line (about 50 feet) and decreased to less than 5 feet as we moved away from the

state line into New Mexico. Texas pumping induced approximately 5 feet drawdown in the vicinity of Clovis and east of Portales Valley.

The effects of New Mexico pumping on Texas water levels were also examined by performing a historical simulation with no pumping in Texas. New Mexico pumping induced 35 to 50 feet drawdown in Texas near the New Mexico-Texas state line and decreased to 5 feet 3 to 5 miles east of the state line. Thus, the greatest effects from pumping in Texas, New Mexico, or both are observed in areas adjacent to the New Mexico-Texas state line.

A calibrated model is a valuable tool for water rights administration and as an aid to planning efforts. As a tool for water rights administration, the model can provide an estimate of the amount of water remaining for appropriation and to calculate the effects of proposed wells upon existing water rights. As an aid to local planning efforts, the model can be used as a tool for calculating regional effects and evaluating different predictive scenarios of future groundwater pumpage in both New Mexico and Texas. However, a more important use of the model may be to increase our awareness that substantial portions of the basin may be de-watered within a decade and to evaluate options for prolonging the life expectancy of the limited water resource within this region.

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IX. APPENDICES

Appendix A
Steady-State Constant Head Flow (“-“ is Discharge, “+” is Recharge)

Layer	Row	Column	Flow(cfd)
1	1	19	106.363
1	1	20	-518.686
1	1	21	-61.3079
1	1	22	41.5812
1	1	23	665.292
1	1	24	1372.24
1	1	25	792.859
1	1	26	612.452
1	1	27	861.021
1	1	28	2242.61
1	1	29	3511.43
1	1	30	3296.49
1	1	31	1705.11
1	1	32	-535.559
1	1	33	-1681.18
1	1	34	-1918.68
1	1	35	-1790.27
1	1	36	-1490.49
1	1	37	-1265.84
1	1	38	-782.505
1	1	39	-107.070
1	1	40	490.054
1	1	41	5681.85
1	1	42	4099.43
1	1	43	7661.44
1	1	44	2065.22
1	1	45	1749.57
1	1	46	963.127
1	1	47	911.965
1	1	48	2422.07
1	1	49	6735.62
1	1	50	10259.5
1	1	51	14502.4
1	1	52	16039.8
1	1	53	14714.3
1	1	54	11552.6
1	1	55	6727.05
1	1	56	827.492
1	1	57	99.8336
1	2	18	1613.02
1	2	58	-77959.2
1	3	17	6546.55
1	3	58	-57383.7
1	4	17	6943.39
1	4	58	-47915.8
1	5	8	-19.8885
1	5	9	751.065
1	5	10	1042.99
1	5	11	1816.04
1	5	12	7.55226
1	5	13	165.225
1	5	14	1274.46
1	5	15	2113.23
1	5	16	19829.9
1	5	58	-48482.6
1	6	6	175.804
1	6	7	11076.2
1	6	58	-50198.1
1	7	4	105.516
1	7	5	1662.47
1	7	58	-60178.6
1	8	2	1480.22

Layer	Row	Column	Flow (cfd)
1	8	3	1580.67
1	8	58	-70522.6
1	9	1	-2930.71
1	9	58	-80128.3
1	10	1	-7995.44
1	10	58	-77656.4
1	11	1	-8849.48
1	11	58	-79240.5
1	12	1	-2614.56
1	12	58	-80370.3
1	13	1	-7145.60
1	13	58	-77774.5
1	14	1	-7732.76
1	14	58	-82908.9
1	15	1	-4735.49
1	15	58	-79553.2
1	16	1	-402.967
1	16	58	-84590.0
1	17	1	11932.4
1	17	58	-79021.4
1	18	1	15137.1
1	18	58	-71056.7
1	19	1	15055.5
1	19	58	-63224.9
1	20	1	10824.5
1	20	58	-64903.5
1	21	1	22692.8
1	21	58	-56456.8
1	22	1	32146.0
1	22	58	-65278.2
1	23	1	28304.8
1	23	58	-66974.5
1	24	1	24475.8
1	24	58	-73714.2
1	25	1	23753.0
1	25	58	-76108.2
1	26	1	26613.4
1	26	58	-84010.8
1	27	1	25475.4
1	27	58	-78963.1
1	28	1	11315.4
1	28	58	-80323.2
1	29	1	7394.46
1	29	58	-93515.6
1	30	1	5560.63
1	30	58	-99015.8
1	31	1	-399.805
1	31	58	-118809.
1	32	1	-8998.51
1	32	58	-130231.
1	33	1	-7515.72
1	33	58	-131907.
1	34	1	-5267.99
1	34	58	-131768.
1	35	1	6516.97
1	35	58	-132269.
1	36	1	11157.6
1	36	58	-128432.
1	37	1	7161.82
1	37	58	-129188.
1	38	1	3583.83
1	38	58	-135688.
1	39	1	10407.9
1	39	58	-126327.
1	40	1	15477.4
1	40	58	-120405.

Layer	Row	Column	Flow (cfd)
1	41	1	13732.1
1	41	58	-119207.
1	42	1	35459.8
1	42	58	-127690.
1	43	1	33421.0
1	43	58	-119141.
1	44	1	57387.6
1	44	58	-109700.
1	45	1	46574.4
1	45	58	-95919.0
1	46	1	159789.
1	46	58	-78936.6
1	47	1	221647.
1	47	58	-77525.4
1	48	1	138390.
1	48	58	-74147.6
1	49	1	124106.
1	49	58	-70725.4
1	50	1	3248.74
1	50	58	-59526.9
1	51	1	1213.96
1	51	58	-101269.
1	52	1	76.7583
1	52	58	-95585.7
1	53	1	-467.272
1	53	58	-101103.
1	54	1	-2189.42
1	54	58	-105998.
1	55	1	-1311.22
1	55	58	-105635.
1	56	1	327.827
1	56	58	-93119.2
1	57	1	1721.40
1	57	58	-75086.4
1	58	1	2898.32
1	58	58	-61567.2
1	59	1	3124.32
1	59	58	-54293.2
1	60	1	2247.18
1	60	58	-59225.8
1	61	2	3576.26
1	61	58	-19787.9
1	62	2	1869.87
1	62	58	-24044.0
1	63	2	6914.50
1	63	58	-27534.2
1	64	3	2221.28
1	64	58	-30909.7
1	65	4	161.171
1	65	5	2841.02
1	65	58	-34620.3
1	66	6	-16.1938
1	66	7	2520.39
1	66	58	-40226.8
1	67	8	39.9469
1	67	9	4973.62
1	67	58	-45548.0
1	68	10	177.025
1	68	11	3682.26
1	68	58	-48461.7
1	69	12	188.660
1	69	13	2735.09
1	69	58	-37530.2
1	70	14	-9.39774
1	70	15	-626.463
1	70	16	-1643.24

Layer	Row	Column	Flow (cfd)
1	70	58	-37156.3
1	71	17	-690.036
1	71	18	-1314.00
1	71	58	-37403.1
1	72	19	-3109.30
1	72	22	1212.11
1	72	23	2844.80
1	72	24	2638.28
1	72	25	679.197
1	72	26	6270.40
1	72	58	-39674.4
1	73	27	-739.228
1	73	28	-383.885
1	73	29	-837.744
1	73	30	-877.093
1	73	31	-394.989
1	73	32	91.0077
1	73	33	-150.482
1	73	34	-227.618
1	73	35	-407.645
1	73	36	-813.428
1	73	37	-1746.90
1	73	38	4421.57
1	73	58	-29727.2
1	74	39	4951.30
1	74	40	3519.42
1	74	41	1670.35
1	74	42	-149.122
1	74	43	-1973.57
1	74	44	-4244.68
1	74	45	-5912.95
1	74	46	-6490.58
1	74	47	-8353.84
1	74	48	-7560.76
1	74	49	-6911.91
1	74	50	-6145.44
1	74	51	-5843.70
1	74	52	-5574.21
1	74	53	-5370.29
1	74	54	-5192.95
1	74	55	-4626.04
1	74	56	-3849.68
1	74	57	-3614.21

Appendix B

MODFLOW INPUT FILES

MODFLOW BASIC FILE

[illegible]

March 4, 1999
CPB7.WPD

March 4, 1999
CPB7.WPD

9.9999e+02

March 4, 1999
CPB7.WPD

0.4078E+04 0.4053E+04 0.4036E+04 0.4023E+04 0.4011E+04 0.4000E+04 0.3989E+04 0.3979E+04
 0.1000E+04 0.1000E+04 0.1000E+04 0.1000E+04 0.1000E+04 0.4650E+04 0.4648E+04 0.4619E+04 0.4586E+04 0.4573E+04
 0.4558E+04 0.4542E+04 0.4526E+04 0.4508E+04 0.4487E+04 0.4464E+04 0.4440E+04 0.4424E+04 0.4413E+04 0.4404E+04
 0.4399E+04 0.4397E+04 0.4395E+04 0.4385E+04 0.4377E+04 0.4370E+04 0.4363E+04 0.4355E+04 0.4343E+04 0.4326E+04
 0.4293E+04 0.4265E+04 0.4238E+04 0.4213E+04 0.4205E+04 0.4199E+04 0.4191E+04 0.4183E+04 0.4174E+04 0.4163E+04
 0.4155E+04 0.4152E+04 0.4149E+04 0.4145E+04 0.4141E+04 0.4136E+04 0.4129E+04 0.4120E+04 0.4112E+04 0.4102E+04
 0.4084E+04 0.4060E+04 0.4043E+04 0.4029E+04 0.4016E+04 0.4002E+04 0.3988E+04 0.3977E+04
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 0.4192E+04 0.4190E+04 0.4188E+04 0.4185E+04 0.4182E+04 0.4179E+04 0.4175E+04 0.4172E+04 0.4168E+04 0.4164E+04
 0.4160E+04 0.4156E+04 0.4152E+04 0.4149E+04 0.4145E+04 0.4142E+04 0.4136E+04 0.4126E+04 0.4117E+04 0.4097E+04
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 0.4524E+04 0.4502E+04 0.4476E+04 0.4448E+04 0.4416E+04 0.4372E+04 0.4343E+04 0.4338E+04 0.4334E+04 0.4331E+04
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 0.4197E+04 0.4194E+04 0.4191E+04 0.4188E+04 0.4184E+04 0.4180E+04 0.4176E+04 0.4173E+04 0.4169E+04 0.4165E+04
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 0.4062E+04 0.4047E+04 0.4033E+04 0.4019E+04 0.4004E+04 0.3992E+04 0.3984E+04 0.3977E+04
 0.4593E+04 0.4596E+04 0.4599E+04 0.4627E+04 0.4626E+04 0.4618E+04 0.4605E+04 0.4586E+04 0.4558E+04 0.4531E+04
 0.4510E+04 0.4486E+04 0.4460E+04 0.4431E+04 0.4397E+04 0.4352E+04 0.4323E+04 0.4318E+04 0.4313E+04 0.4308E+04
 0.4302E+04 0.4295E+04 0.4286E+04 0.4277E+04 0.4268E+04 0.4258E+04 0.4247E+04 0.4235E+04 0.4224E+04 0.4215E+04
 0.4208E+04 0.4202E+04 0.4197E+04 0.4193E+04 0.4188E+04 0.4183E+04 0.4179E+04 0.4174E+04 0.4170E+04 0.4166E+04
 0.4162E+04 0.4158E+04 0.4153E+04 0.4149E+04 0.4146E+04 0.4142E+04 0.4135E+04 0.4123E+04 0.4102E+04 0.4077E+04
 0.4058E+04 0.4043E+04 0.4028E+04 0.4014E+04 0.4000E+04 0.3989E+04 0.3982E+04 0.3976E+04
 0.4565E+04 0.4573E+04 0.4580E+04 0.4588E+04 0.4598E+04 0.4596E+04 0.4586E+04 0.4569E+04 0.4543E+04 0.4518E+04
 0.4498E+04 0.4474E+04 0.4447E+04 0.4417E+04 0.4382E+04 0.4336E+04 0.4306E+04 0.4301E+04 0.4296E+04 0.4291E+04
 0.4286E+04 0.4280E+04 0.4273E+04 0.4266E+04 0.4259E+04 0.4250E+04 0.4242E+04 0.4233E+04 0.4224E+04 0.4217E+04
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 0.4080E+04 0.4071E+04 0.4062E+04 0.4050E+04 0.4034E+04 0.4018E+04 0.4005E+04 0.3995E+04 0.3985E+04 0.3975E+04
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 0.4224E+04 0.4221E+04 0.4212E+04 0.4197E+04 0.4185E+04 0.4175E+04 0.4166E+04 0.4158E+04 0.4146E+04 0.4121E+04
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 0.3959E+04 0.3949E+04 0.3940E+04 0.3930E+04 0.3921E+04 0.3912E+04 0.3902E+04 0.3893E+04 0.3884E+04 0.3875E+04
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 0.0000E+00 0.4210E+04 0.4204E+04 0.4198E+04 0.4191E+04 0.4183E+04 0.4174E+04 0.4163E+04 0.4148E+04 0.4128E+04
 0.4107E+04 0.4087E+04 0.4067E+04 0.4045E+04 0.4024E+04 0.4007E+04 0.3995E+04 0.3983E+04 0.3972E+04 0.3962E+04
 0.3952E+04 0.3942E+04 0.3932E+04 0.3922E+04 0.3913E+04 0.3904E+04 0.3895E+04 0.3886E+04 0.3877E+04 0.3867E+04
 0.3857E+04 0.3845E+04 0.3834E+04 0.3822E+04 0.3810E+04 0.3796E+04 0.3782E+04 0.3767E+04
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0.4456E+04	0.4437E+04	0.4420E+04	0.4406E+04	0.4390E+04	0.4371E+04	0.4352E+04	0.4334E+04	0.4315E+04	0.4296E+04
0.4278E+04	0.4260E+04	0.4249E+04	0.4240E+04	0.4231E+04	0.4222E+04	0.4212E+04	0.4206E+04	0.4210E+04	0.4200E+04
0.4190E+04	0.4189E+04	0.4190E+04	0.4178E+04	0.4167E+04	0.4157E+04	0.4138E+04	0.4091E+04	0.4063E+04	0.4024E+04
0.3993E+04	0.3963E+04	0.3929E+04	0.3900E+04	0.3889E+04	0.3881E+04	0.3872E+04	0.3862E+04		
0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00
0.0000E+00	0.0000E+00	0.4513E+04	0.4504E+04	0.4491E+04	0.4469E+04	0.4388E+04	0.4375E+04	0.4362E+04	0.4352E+04
0.4417E+04	0.4431E+04	0.4414E+04	0.4396E+04	0.4377E+04	0.4358E+04	0.4340E+04	0.4321E+04	0.4302E+04	0.4284E+04
0.4268E+04	0.4258E+04	0.4249E+04	0.4240E+04	0.4230E+04	0.4221E+04	0.4212E+04	0.4218E+04	0.4212E+04	0.4202E+04
0.4202E+04	0.4203E+04	0.4165E+04	0.4158E+04	0.4150E+04	0.4132E+04	0.4140E+04	0.4116E+04	0.4079E+04	0.4050E+04
0.4011E+04	0.3978E+04	0.3951E+04	0.3920E+04	0.3897E+04	0.3890E+04	0.3880E+04	0.3870E+04		
0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00
0.4533E+04	0.4525E+04	0.4516E+04	0.4497E+04	0.4411E+04	0.4399E+04	0.4386E+04	0.4370E+04	0.4355E+04	0.4342E+04
0.4331E+04	0.4386E+04	0.4402E+04	0.4383E+04	0.4364E+04	0.4346E+04	0.4327E+04	0.4308E+04	0.4290E+04	0.4276E+04
0.4267E+04	0.4257E+04	0.4248E+04	0.4239E+04	0.4199E+04	0.4190E+04	0.4195E+04	0.4222E+04	0.4214E+04	0.4215E+04
0.4216E+04	0.4217E+04	0.4091E+04	0.4078E+04	0.4066E+04	0.4127E+04	0.4135E+04	0.4114E+04	0.4096E+04	0.4066E+04
0.4030E+04	0.3993E+04	0.3962E+04	0.3932E+04	0.3913E+04	0.3900E+04	0.3892E+04	0.3885E+04		
0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.4546E+04
0.4540E+04	0.4523E+04	0.4432E+04	0.4421E+04	0.4410E+04	0.4398E+04	0.4382E+04	0.4364E+04	0.4348E+04	0.4332E+04
0.4400E+04	0.4408E+04	0.4389E+04	0.4370E+04	0.4352E+04	0.4333E+04	0.4314E+04	0.4296E+04	0.4284E+04	0.4275E+04
0.4266E+04	0.4257E+04	0.4247E+04	0.4238E+04	0.4117E+04	0.4100E+04	0.4104E+04	0.4119E+04	0.4198E+04	0.4204E+04
0.4113E+04	0.4092E+04	0.4081E+04	0.4069E+04	0.4057E+04	0.4046E+04	0.4104E+04	0.4115E+04	0.4100E+04	0.4078E+04
0.4045E+04	0.4008E+04	0.3972E+04	0.3944E+04	0.3927E+04	0.3919E+04	0.3910E+04	0.3900E+04		
0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.4568E+04	0.4565E+04	0.4573E+04
0.4550E+04	0.4437E+04	0.4422E+04	0.4415E+04	0.4404E+04	0.4395E+04	0.4371E+04	0.4348E+04	0.4331E+04	0.4319E+04
0.4389E+04	0.4395E+04	0.4376E+04	0.4358E+04	0.4339E+04	0.4320E+04	0.4303E+04	0.4293E+04	0.4284E+04	0.4274E+04
0.4265E+04	0.4256E+04	0.4247E+04	0.4131E+04	0.4109E+04	0.4095E+04	0.4095E+04	0.4093E+04	0.4091E+04	0.4089E+04
0.4086E+04	0.4080E+04	0.4070E+04	0.4059E+04	0.4047E+04	0.4036E+04	0.4025E+04	0.4090E+04	0.4071E+04	0.3986E+04
0.4055E+04	0.4015E+04	0.3978E+04	0.3956E+04	0.3941E+04	0.3937E+04	0.3929E+04	0.3900E+04		
0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.0000E+00	0.4589E+04	0.4589E+04	0.4598E+04	0.4479E+04	0.4460E+04
0.4440E+04	0.4420E+04	0.4407E+04	0.4402E+04	0.4397E+04	0.4375E+04	0.4342E+04	0.4322E+04	0.4310E+04	0.4300E+04
0.4292E+04	0.4349E+04	0.4364E+04	0.4345E+04	0.4326E+04	0.4311E+04	0.4301E+04	0.4292E+04	0.4283E+04	0.4274E+04
0.4265E+04	0.4223E+04	0.4220E+04	0.4124E+04	0.4106E+04	0.4095E+04	0.4091E+04	0.4085E+04	0.4080E+04	0.4078E+04
0.4074E+04	0.4068E+04	0.4058E+04	0.4047E+04	0.4037E+04	0.4027E+04	0.4017E+04	0.4005E+04	0.3993E+04	0.3981E+04
0.4047E+04	0.4007E+04	0.3978E+04	0.3950E+04	0.3950E+04	0.3950E+04	0.3935E+04	0.3900E+04		
0.0000E+00	0.0000E+00	0.0000E+00	0.4610E+04	0.4614E+04	0.4622E+04	0.4512E+04	0.4491E+04	0.4468E+04	0.4443E+04
0.4419E+04	0.4400E+04	0.4376E+04	0.4358E+04	0.4337E+04	0.4319E+04	0.4306E+04	0.4296E+04	0.4283E+04	0.4276E+04
0.4270E+04	0.4342E+04	0.4351E+04	0.4332E+04	0.4319E+04	0.4310E+04	0.4301E+04	0.4292E+04	0.4282E+04	0.4246E+04
0.4152E+04	0.4138E+04	0.4127E+04	0.4114E+04	0.4099E+04	0.4089E+04	0.4083E+04	0.4076E+04	0.4070E+04	0.4067E+04
0.4061E+04	0.4054E+04	0.4046E+04	0.4038E+04	0.4030E+04	0.4020E+04	0.4009E+04	0.3998E+04	0.3987E+04	0.4059E+04
0.4010E+04	0.3966E+04	0.3946E+04	0.3938E+04	0.3928E+04	0.3914E+04	0.3896E+04	0.3871E+04		
0.0000E+00	0.4631E+04	0.4638E+04	0.4619E+04	0.4539E+04	0.4513E+04	0.4490E+04	0.4476E+04	0.4462E+04	0.4437E+04
0.4410E+04	0.4381E+04	0.4351E+04	0.4318E+04	0.4291E+04	0.4281E+04	0.4271E+04	0.4260E+04	0.4251E+04	0.4248E+04
0.4247E+04	0.4245E+04	0.4307E+04	0.4328E+04	0.4285E+04	0.4309E+04	0.4300E+04	0.4291E+04	0.4255E+04	0.4146E+04
0.4131E+04	0.4118E+04	0.4108E+04	0.4100E+04	0.4091E+04	0.4083E+04	0.4075E+04	0.4068E+04	0.4062E+04	0.4057E+04
0.4050E+04	0.4044E+04	0.4038E+04	0.4030E+04	0.4022E+04	0.4014E+04	0.4003E+04	0.3992E+04	0.4071E+04	0.4026E+04
0.3973E+04	0.3939E+04	0.3923E+04	0.3912E+04	0.3892E+04	0.3871E+04	0.3852E+04	0.3826E+04		
0.4488E+04	0.4508E+04	0.4531E+04	0.4625E+04	0.4534E+04	0.4504E+04	0.4482E+04	0.4464E+04	0.4449E+04	0.4425E+04
0.4391E+04	0.4364E+04	0.4333E+04	0.4297E+04	0.4276E+04	0.4259E+04	0.4244E+04	0.4233E+04	0.4225E+04	0.4219E+04
0.4216E+04	0.4215E+04	0.4209E+04	0.4202E+04	0.4194E+04	0.4195E+04	0.4194E+04	0.4185E+04	0.4161E+04	0.4137E+04
0.4116E+04	0.4099E+04	0.4095E+04	0.4089E+04	0.4085E+04	0.4077E+04	0.4070E+04	0.4062E+04	0.4055E+04	0.4048E+04
0.4041E+04	0.4036E+04	0.4030E+04	0.4024E+04	0.4015E+04	0.4006E+04	0.3997E+04	0.3986E+04	0.4040E+04	0.4000E+04
0.3945E+04	0.3925E+04	0.3905E+04	0.3887E+04	0.3862E+04	0.3834E+04	0.3804E+04	0.3787E+04		
0.4460E+04	0.4480E+04	0.4507E+04	0.4539E+04	0.4526E+04	0.4498E+04	0.4475E+04	0.4455E+04	0.4434E+04	0.4408E+04
0.4382E+04	0.4357E+04	0.4326E+04	0.4294E+04	0.4270E+04	0.4246E+04	0.4228E+04	0.4215E+04	0.4204E+04	0.4194E+04
0.4191E+04	0.4190E+04	0.4188E+04	0.4185E+04	0.4181E+04	0.4178E+04	0.4174E+04	0.4164E+04	0.4141E+04	0.4123E+04
0.4102E+04	0.4095E+04	0.4089E+04	0.4083E+04	0.4077E+04	0.4071E+04	0.4063E+04	0.4056E+04	0.4049E+04	0.4042E+04
0.4035E+04	0.4093E+04	0.4100E+04	0.4017E+04	0.4010E+04	0.4001E+04	0.3991E+04	0.4066E+04	0.4019E+04	0.3967E+04
0.3936E+04	0.3916E+04	0.3894E+04	0.3869E+04	0.3845E+04	0.3811E+04	0.3783E+04	0.3765E+04		
0.4445E+04	0.4454E+04	0.4479E+04	0.4508E+04	0.4503E+04	0.4492E+04	0.4465E+04	0.4443E+04	0.4423E+04	0.4398E+04
0.4376E+04	0.4352E+04	0.4323E+04	0.4293E+04	0.4265E+04	0.4240E+04	0.4219E+04	0.4201E+04	0.4189E+04	0.4182E+04
0.4177E+04	0.4174E+04	0.4173E+04	0.4171E+04	0.4168E+04	0.4164E+04	0.4154E+04	0.4143E+04	0.4132E+04	0.4114E+04
0.4095E+04	0.4091E+04	0.4084E+04	0.4078E+04	0.4071E+04	0.4140E+04	0.4148E+04	0.4134E+04	0.4121E+04	0.4109E+04
0.4100E+04	0.4100E+04	0.4104E+04	0.4104E+04	0.4074E+04	0.3995E+04	0.4061E+04	0.4045E+04	0.3994E+04	0.3944E+04
0.3926E+04	0.3906E+04	0.3886E+04	0.3863E+04	0.3836E+04	0.3806E+04	0.3774E+04	0.3744E+04		
0.4445E+04	0.4430E+04	0.4446E+04	0.4475E+04	0.4495E+04	0.4472E+04	0.4452E+04	0.4427E+04	0.4410E+04	0.4390E+04
0.4371E+04	0.4349E+04	0.4321E+04	0.4292E+04	0.4262E+04	0.4234E+04	0.4212E+04	0.4193E+04	0.4179E+04	0.4169E+04
0.4164E+04	0.4159E+04	0.4158E+04	0.4157E+04	0.4155E+04	0.4150E+04	0.4141E+04	0.4130E+04	0.4120E+04	0.4106E+04

0.4093E+04 0.4086E+04 0.4079E+04 0.4072E+04 0.4066E+04 0.4142E+04 0.4121E+04 0.4100E+04 0.4091E+04 0.4089E+04
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MODFLOW ET FILE

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MODFLOW SIP FILE

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0.30001.0000e-03 00.0040e+00 5

MODFLOW WELL FILE FOR THE LAST STRESS PERIOD

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